

***Advanced constitutive
model for bituminous
materials:
a research challenge for
road engineering***



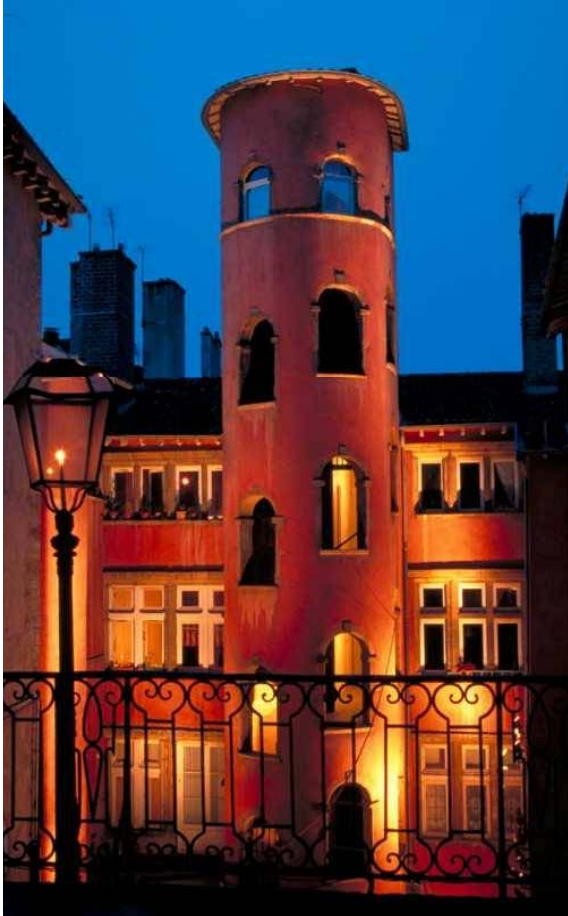
Membre de
UNIVERSITÉ DE LYON



Prof. Hervé Di Benedetto

Outline

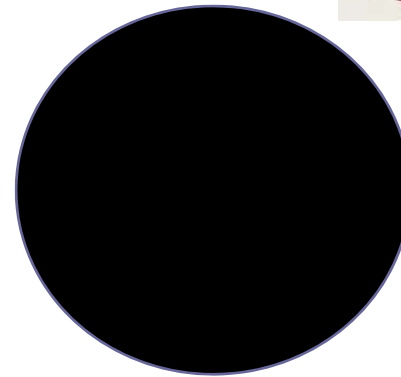
- Introduction: bituminous materials and sollicitations on road
- Types of behaviour for bituminous materials
- The DBN model (thermo-visco-elastoplastic)
- Focus
 - Linear domain: Viscoelasticity (LVE)
 - Time-temperature superposition principle
- Importance of the type of behaviour: examples of numerical simulations
- Conclusion



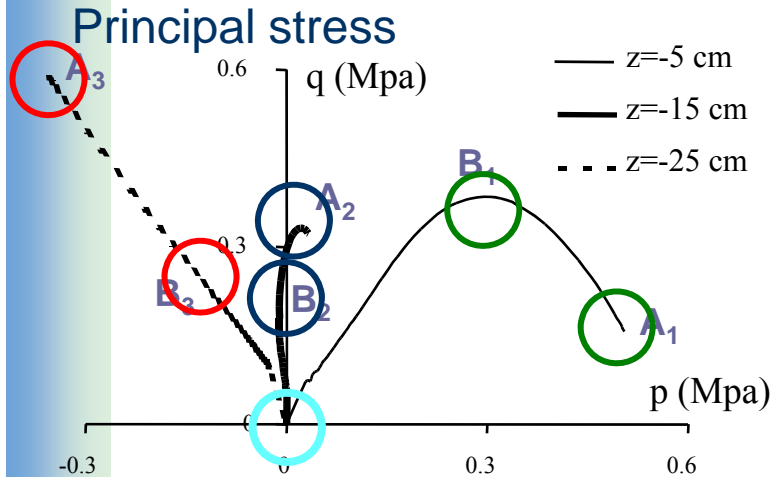
Bitumen, mastic, bituminous mixture

Complex thermo-viscoplastic behaviour

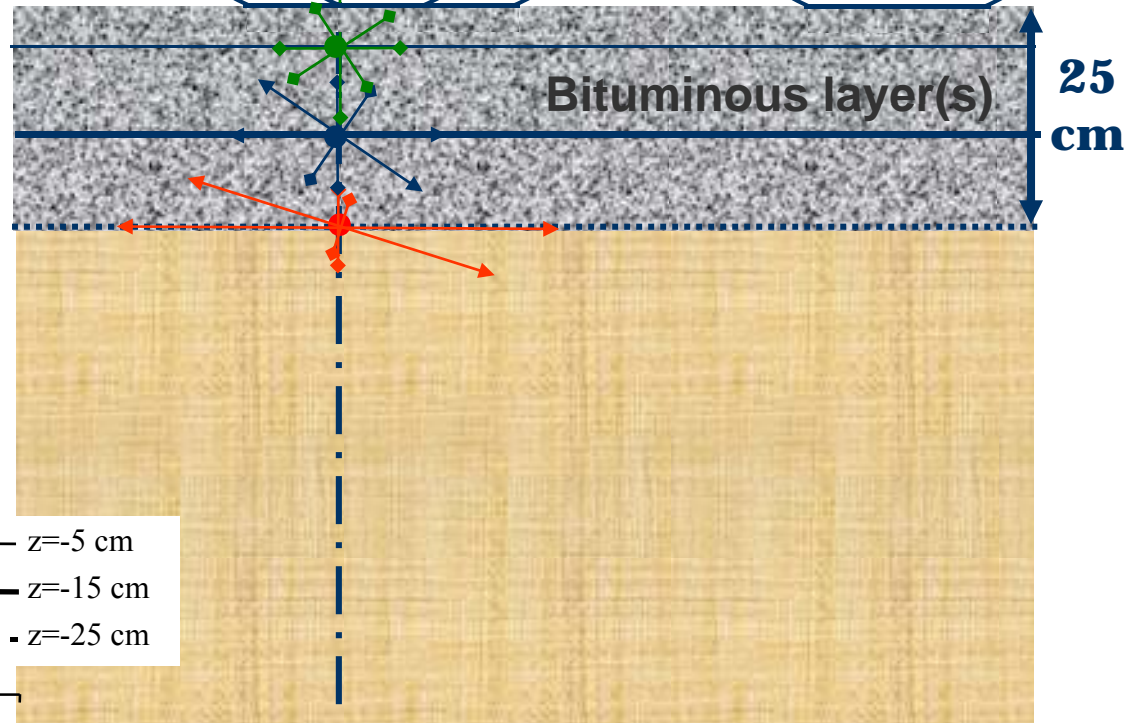
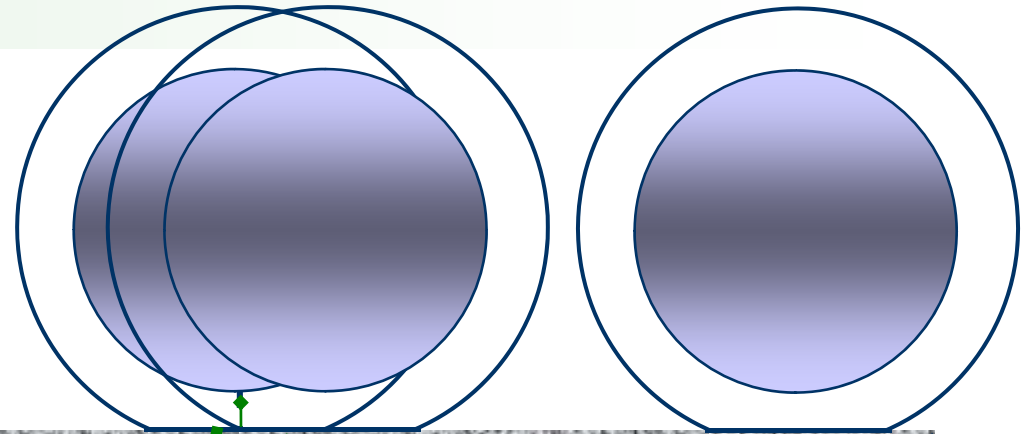
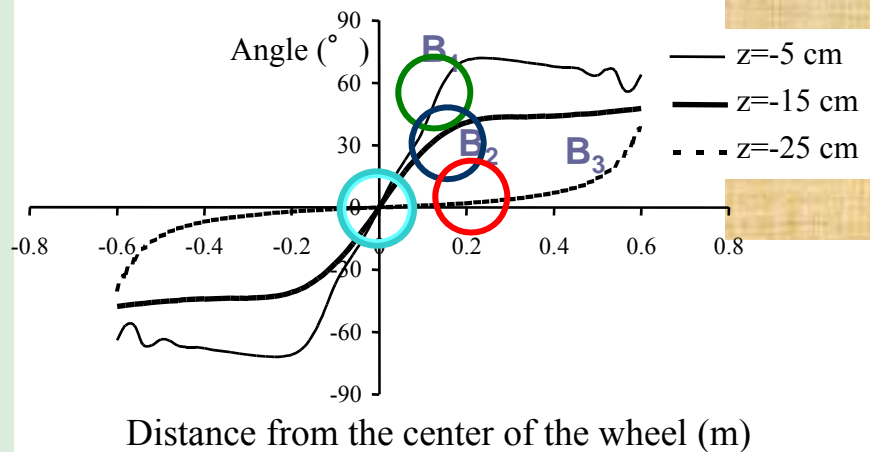
- Bitumen: from fluid to brittle solid
- Mastic : the “glue”
 - Bitumen + fines ($< 100\mu\text{m}$)
- Bituminous mixture : used on road
 - Aggregates: 80% to 85% in volume (92% to 96% in weight)
 - Bitumen: 12% to 20% in volume (4% to 8% in weight)



Stress path In surface layer(s)

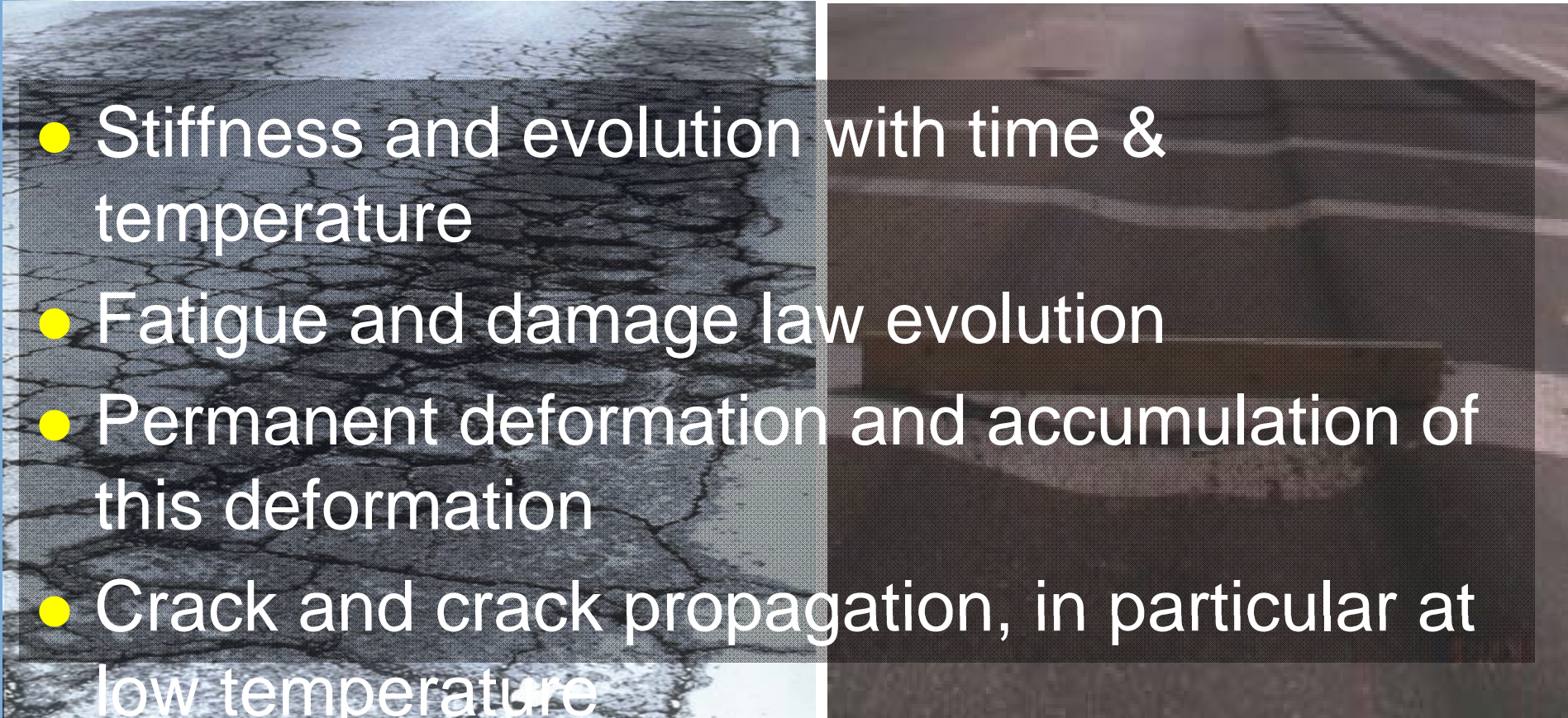


Rotation of axes



**Cycles &
Rotation of axes**

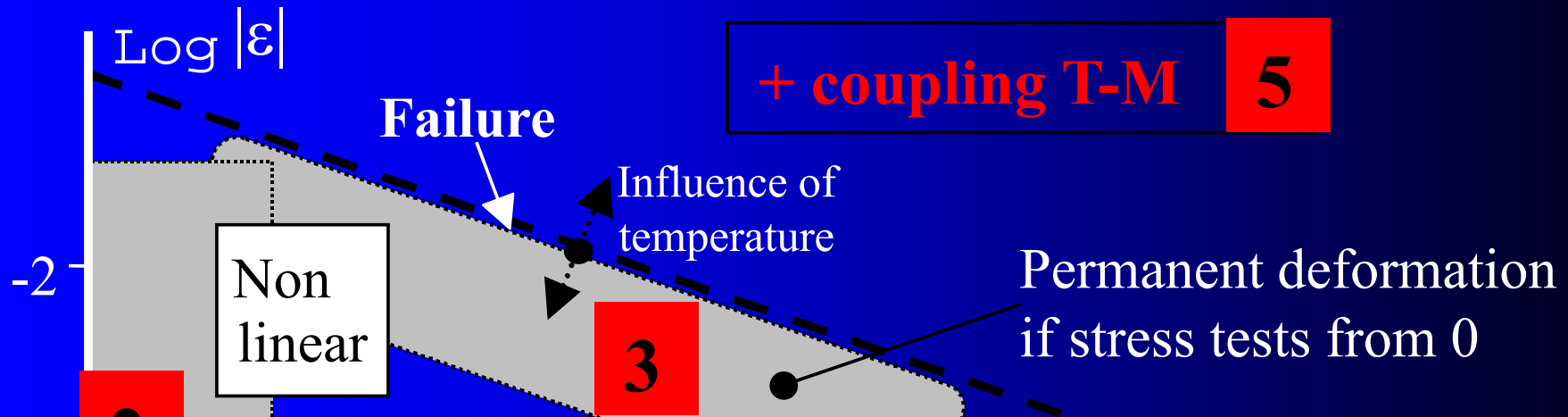
Important aspects for bituminous layers

- 
- Stiffness and evolution with time & temperature
 - Fatigue and damage law evolution
 - Permanent deformation and accumulation of this deformation
 - Crack and crack propagation, in particular at low temperature

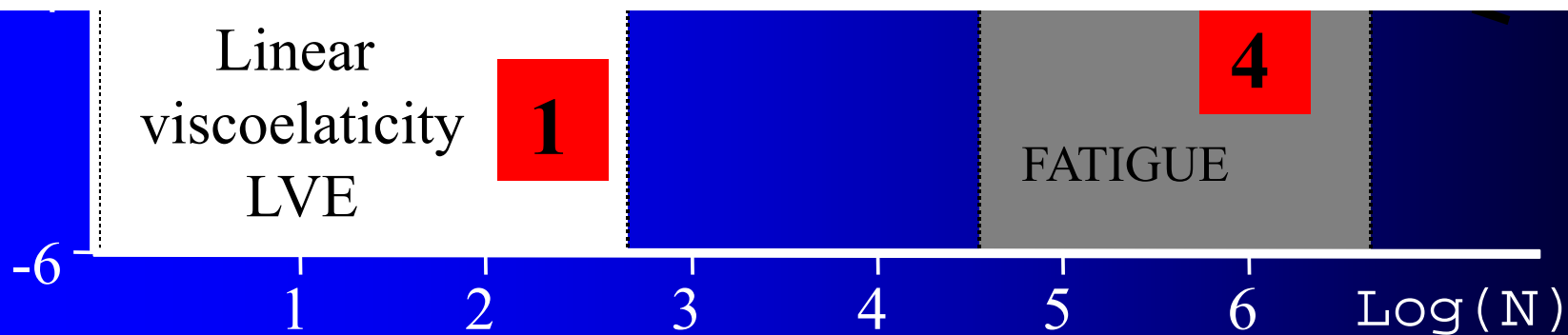
→ different “types or domains” of behaviour for bituminous mixtures

Domains of behaviour

(Di Benedetto 90)

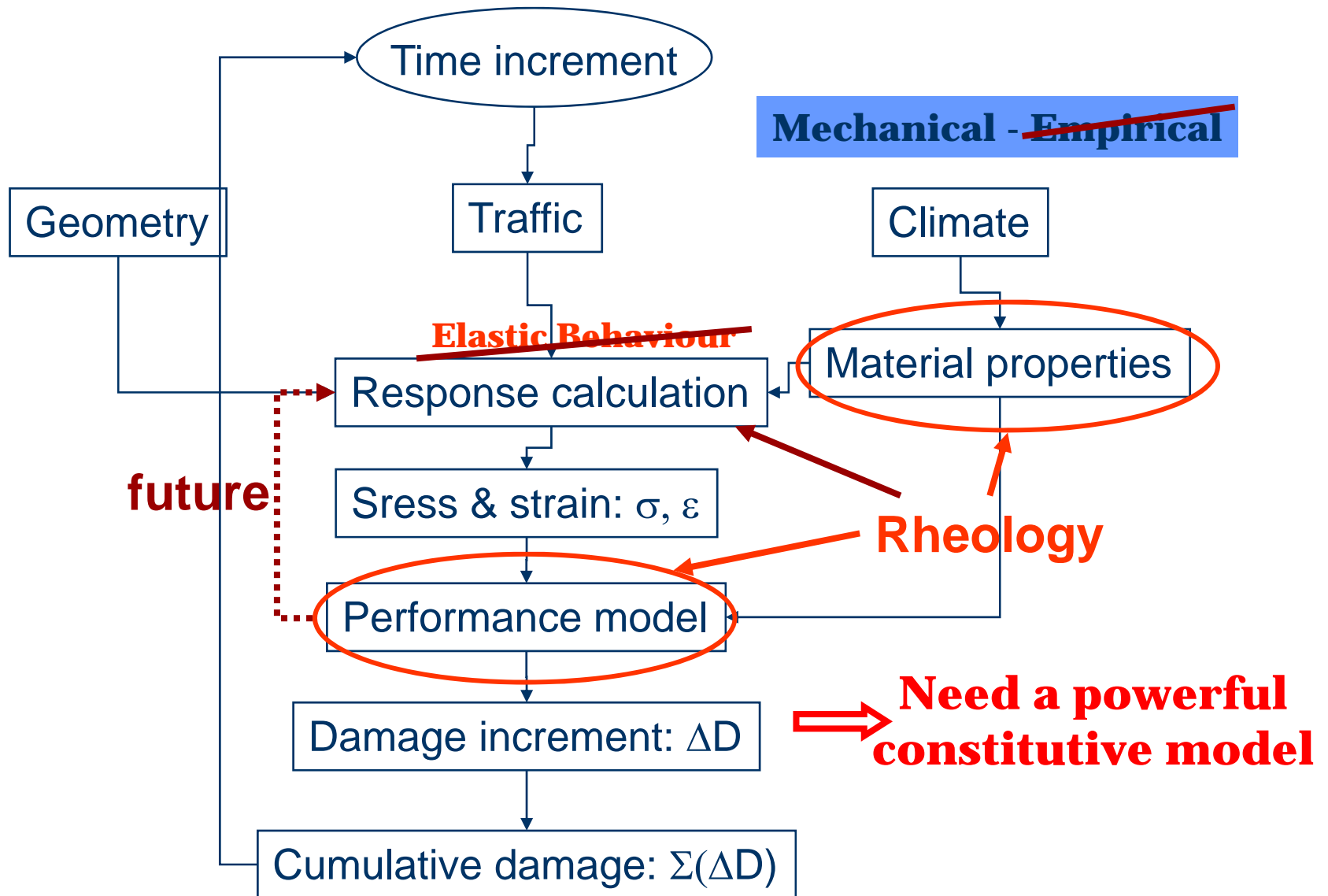


Always great influence of Temperature and loading rate



Bituminous mixtures

Fundamental design method



Behaviour and associated phenomena

- Linear viscoelasticity
- Non linearity
- Fatigue
- Healing
- Thixotropy
- Crack propagation
- Permanent deformation
- Brittle failure
- Viscoplastic flow
- Thermo-mechanical couplin

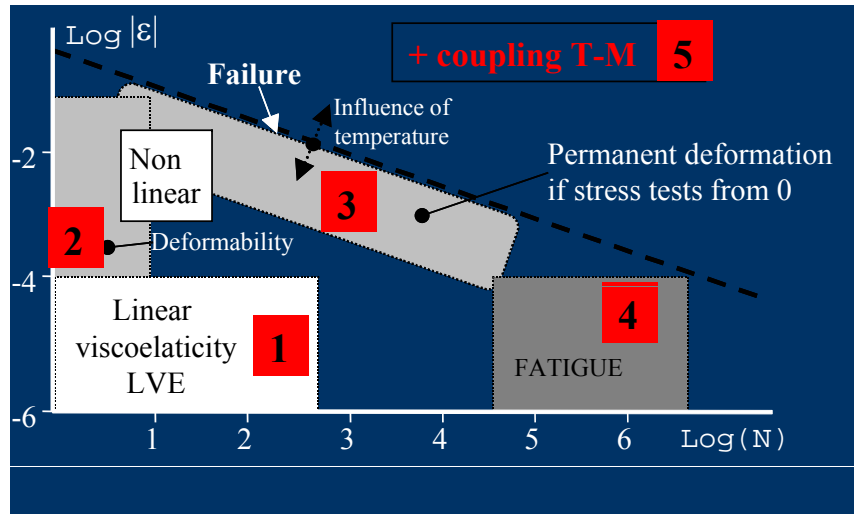
1, 2

4

3

2, 3

3, 5



• 3 D formalism /one D

TSRS Test 1,2,3,5

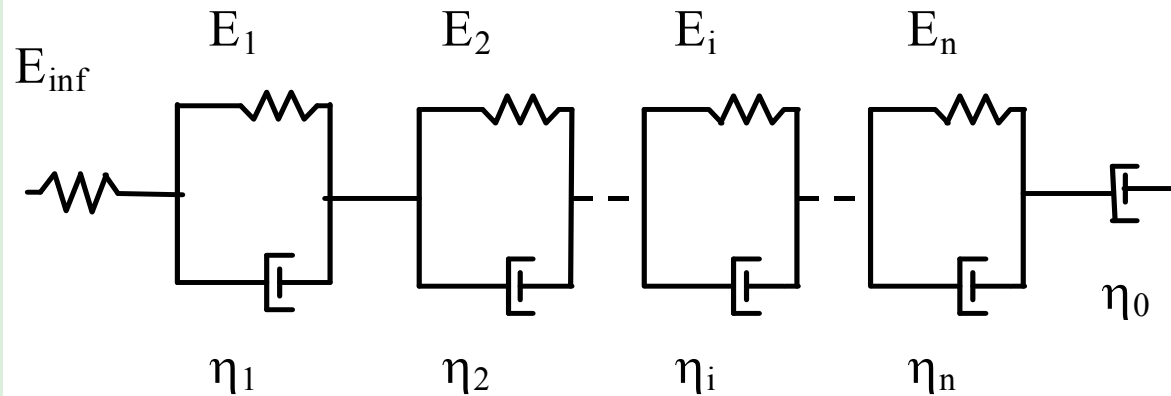
→ Calculation stress path in road

DBN (Di Benedetto – Neifar) Model: Thermo-visco plastic

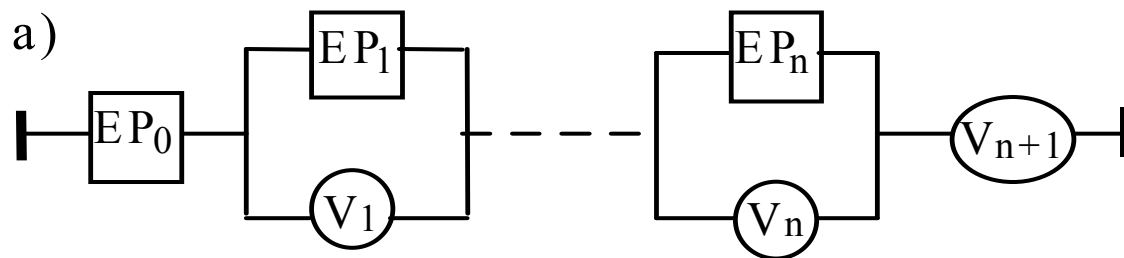
- Introduces non linearity and irreversibility but gives a linear behaviour in the small strain domain (asymptotic behaviour)
- Respects the time – temperature superposition principle (even in the non linear domain)

*One-dimensional formalism of the
DBN Model*

Generalisation of generalised KV body



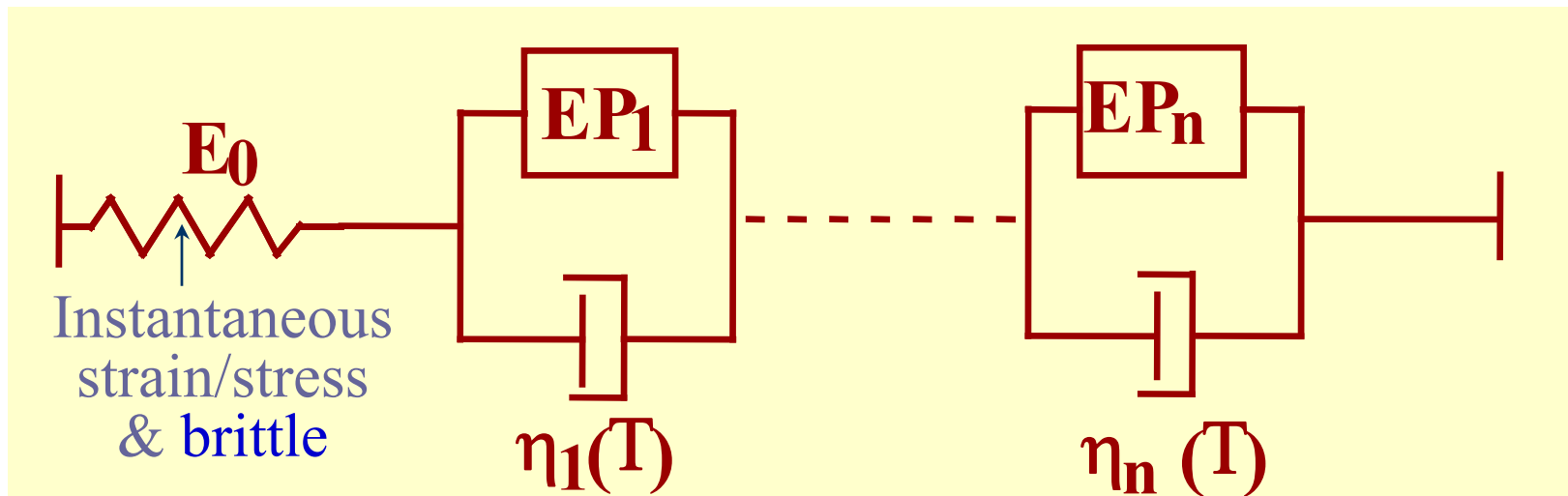
Generalisation
For any material



→ Choice of EP and V for mixes

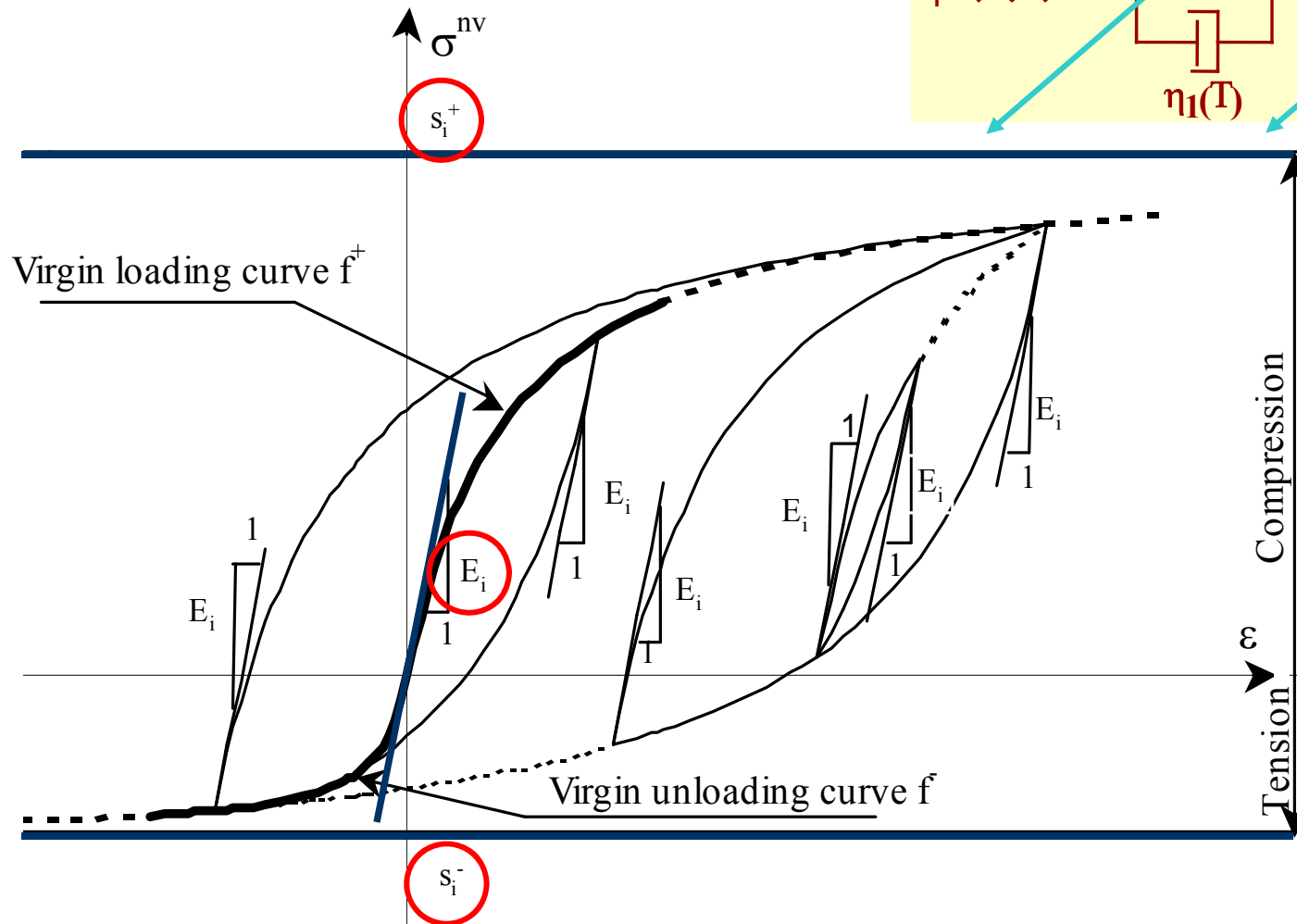
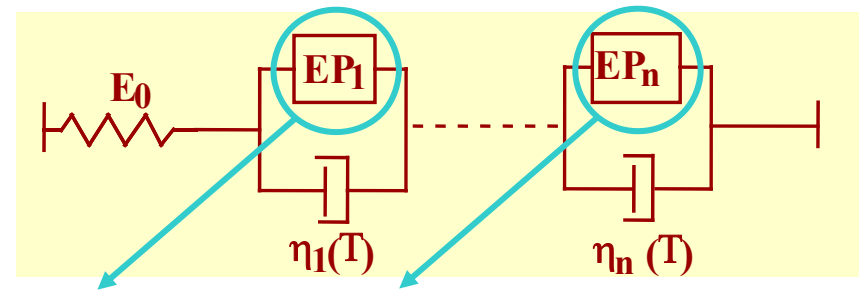
Model For Bituminous mixtures

DBN model (Di Benedetto, Neifar)



Each EP body behaves as a non cohesive granular material

Behaviour of EP_i body: generalisation of the Masing rule



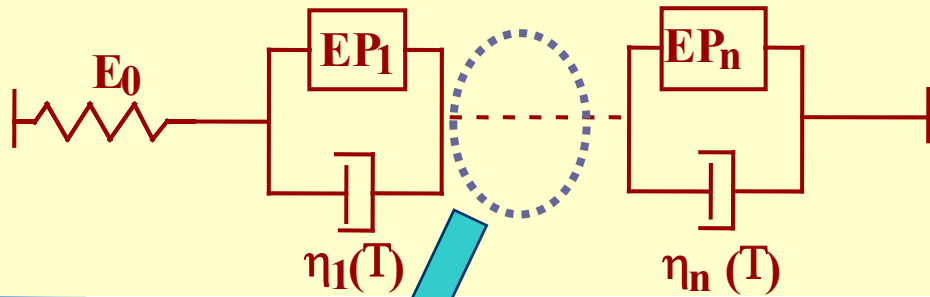
Each EP_i :

E_i

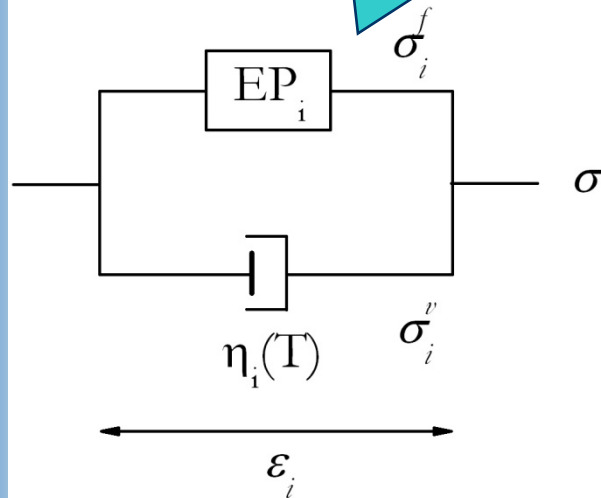
s_i^+

$s_i^- = -k s_i^+$

*Three-dimensional formalism of the
DBN Model*



3 D formalism



- Elastoplastic 3D model for EP_i
- For viscous branch: only one scalar equation and a mapping rule
 - Same equation 1D with:

$$\sigma^v \rightarrow \|\sigma^v\| \quad \text{and} \quad \dot{\varepsilon}^{vp} \rightarrow \|\dot{\varepsilon}^{vp}\|$$
 - Mapping rule : direction of $d(\sigma^f)$

Model calibration

Calibration for small strain

- ◆ Linear viscoelastic (LVE) behaviour
- ◆ Complex modulus tests $\neq \dot{\epsilon}$ & $\neq T$
- Determination of moduli E_j , G_i and viscosities η_j

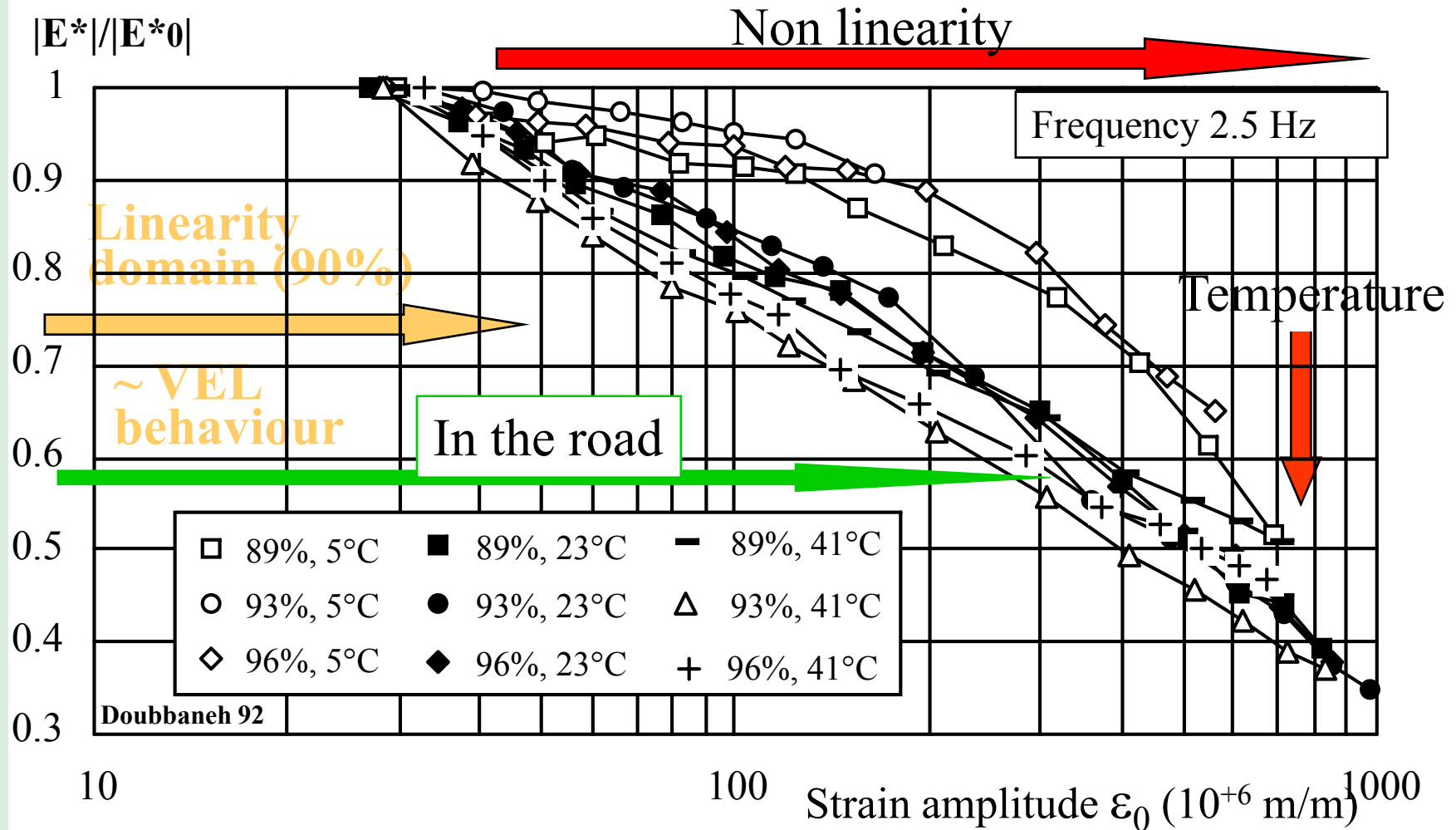
Calibration at failure (in the ductile domain)

- ◆ Viscoplastic behavior $\neq \dot{\epsilon}$ & $\neq T$
- ◆ Failure criterion (Di Benedetto)
- Determination of **Not treated**

Calibration at failure (in the brittle domain)

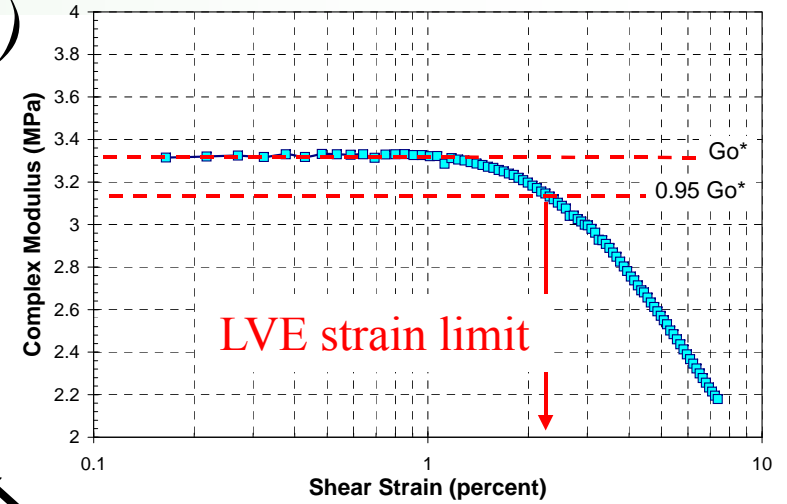
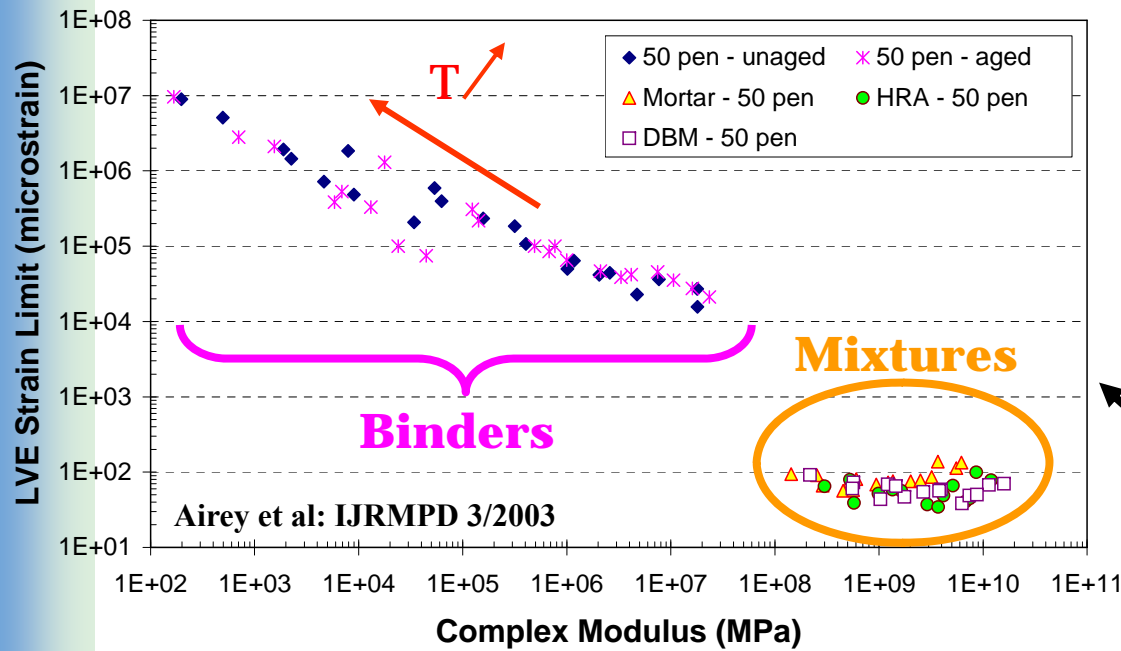
- ◆ Brittle failure at low temperature / high rate
- ◆ Failure criterion $\neq T$
- Determination of brittle limits

Linearity domain (Bituminous mix)

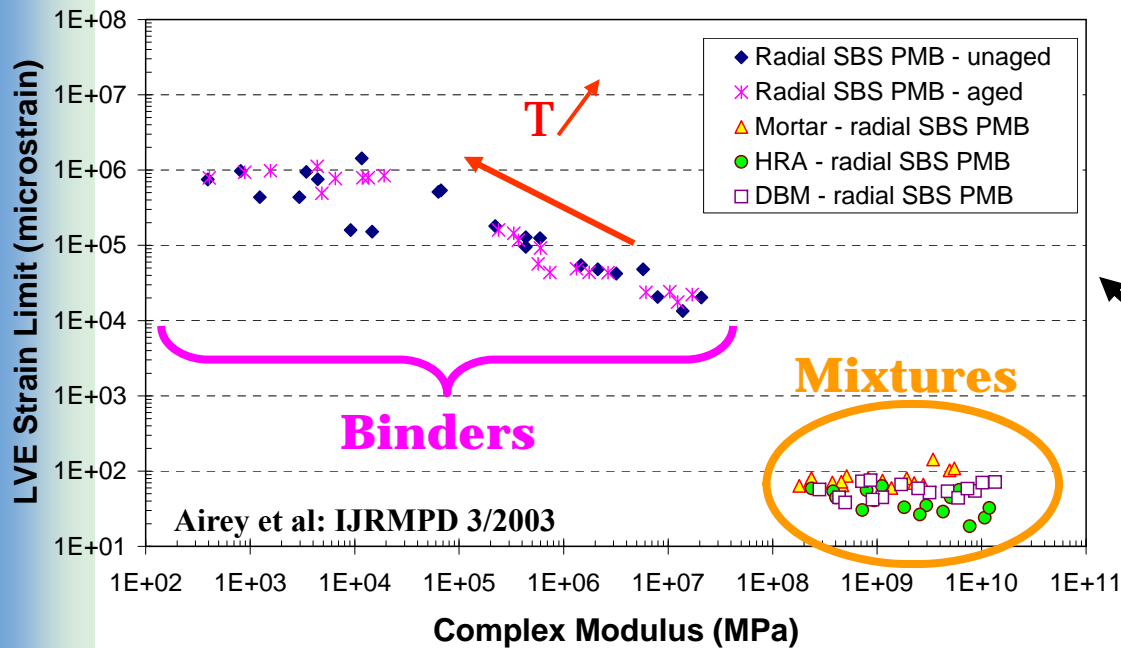


Tension/compression cyclic tests at same frequency and different ϵ_0

Linearity domain (Binders & mixes)

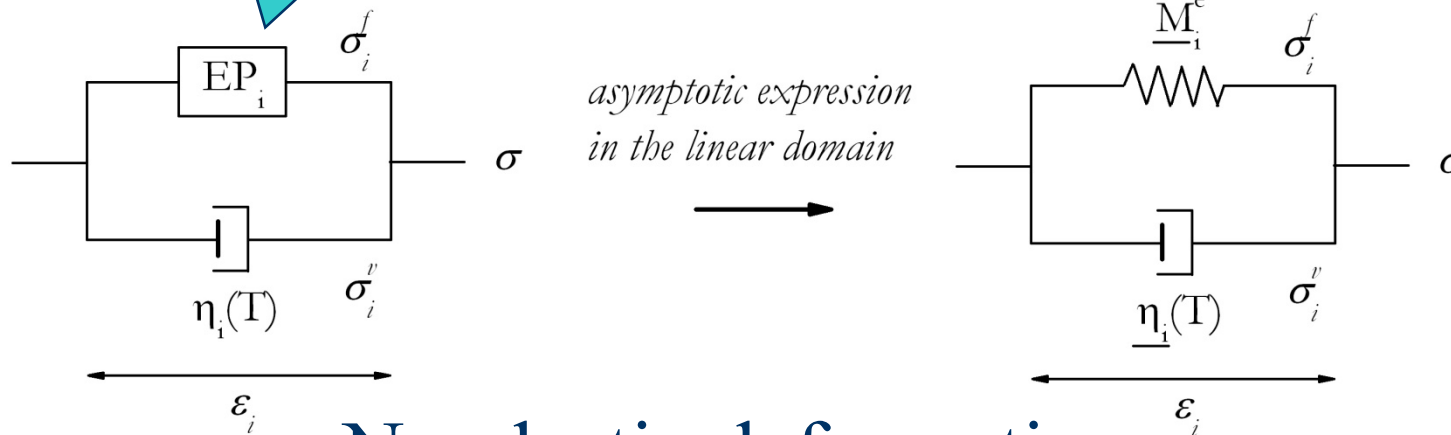
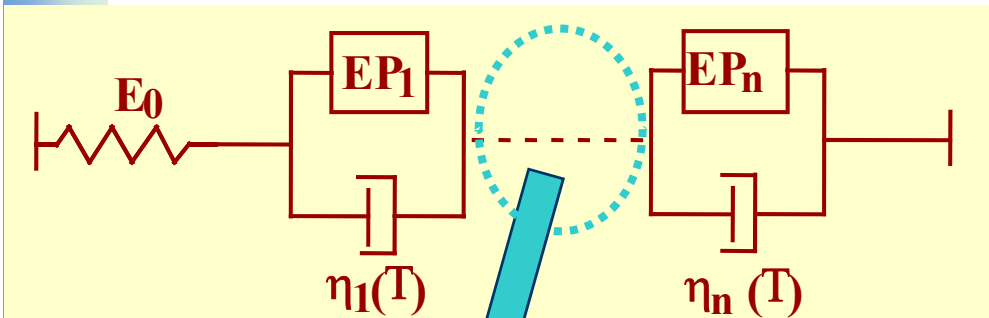


LVE strain limits for 50 pen bitumen and mixtures



LVE strain limits for radial SBS PMB and mixtures

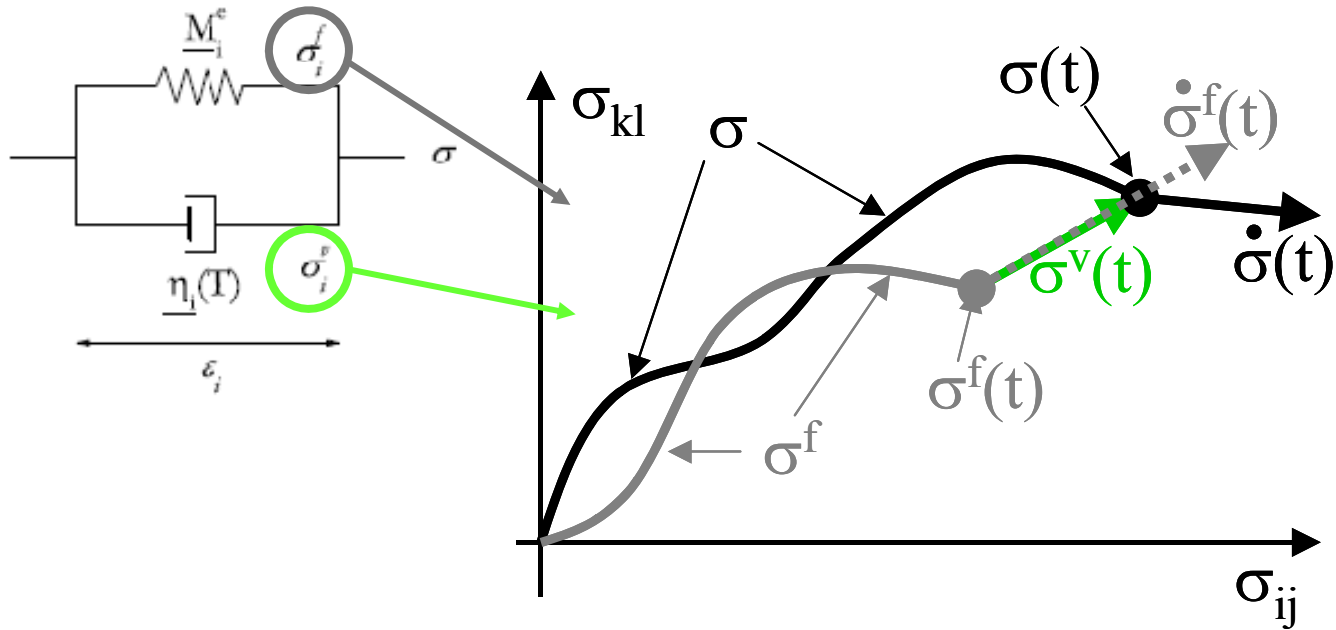
Linear case (small strain domain)



No plastic deformation

→ elastic compliance tensor M_i^e

Mapping rule (linear case)



Isotach case
(Bituminous materials)

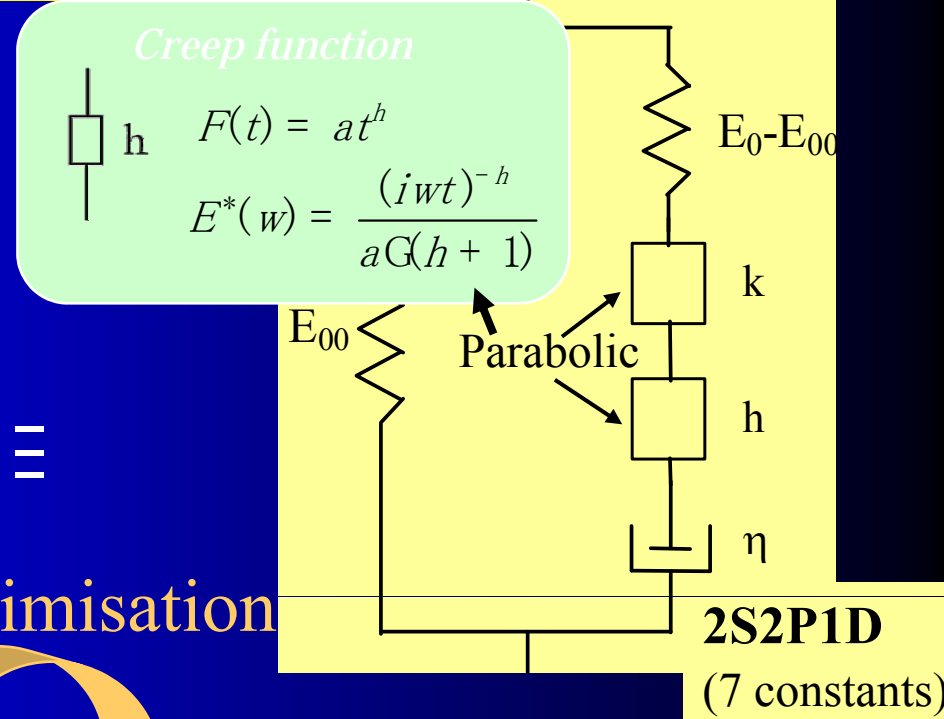
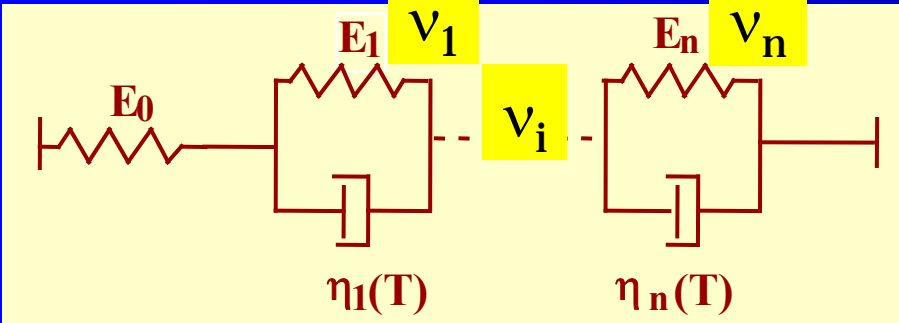
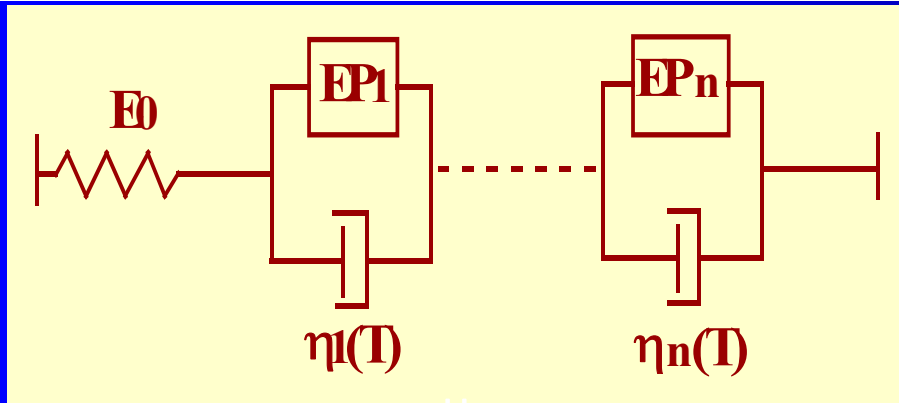
σ^v and $d\sigma^f$ have the
same direction

Calibration in the small strain domain : linear viscoelasticity

- Sinusoidal loading → interpretation in the frequency domain : Complex parameters (E^* , G^* , ν^*)

Calibration for the small strain domain for E (Linear Visco-Elasticity)

3 Dim case



$$E^* = \left(\frac{1}{E_0} + \sum_{j=1}^{10} \frac{1}{E_j + i\eta_j(T)\omega} \right)^{-1}$$

Optimisation

$$E^*(i\omega\tau) = E_{00} + \frac{E_0 - E_{00}}{1 + \delta(i\omega\tau)^{-k} + (i\omega\tau)^{-h} + (i\omega\beta\tau)^{-1}}$$

→ +2

$$\eta_j(T) = \eta_j(T_0) a_T \Rightarrow \tau(T) = \tau_0(T_0) a_T$$

Time-Temperature principle (W.L.F.)

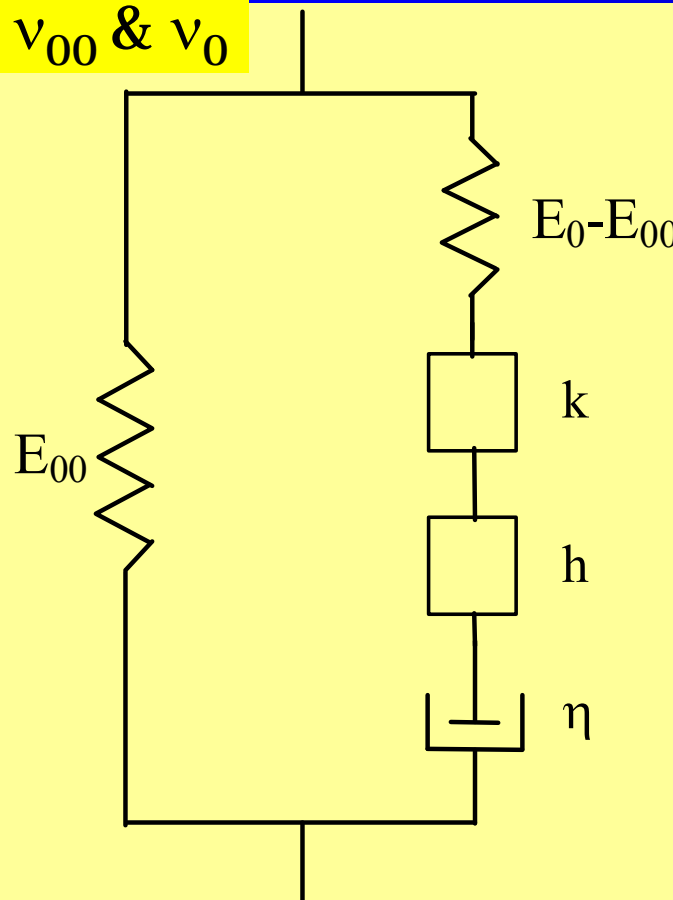
2S2P1D in 3Dim model

(Di Benedetto et al, 2007)

$$E^*(i\omega\tau) = E_0 + \frac{E_\infty - E_0}{1 + \delta(i\omega\tau)^{-k} + (i\omega\tau)^{-h} + (i\omega\beta\tau)^{-1}}$$

$$\nu^*(i\omega\tau) = \nu_0 + \frac{\nu_\infty - \nu_0}{1 + \delta(i\omega\tau_v)^{-k} + (i\omega\tau_v)^{-h} + (i\omega\beta\tau_v)^{-1}}$$

ν_{00} & ν_0



E_{00} , E_0 , ν_{00} , ν_0 , δ , τ , η , h , k & time-temperature superposition principle (C_1 & C_2)

→ 11 constants

- ◆ modelling of binders, mastics & mixes
- ◆ allows the introduction of a prediction formula providing the mix complex modulus and mix Poisson's Ratio from binder ones

→ No simple analytical expression in the time domain

Two devices for bituminous materials: mixes, mastics & bitumen



T/C test (2 types)

H=160mm, $\phi_{\text{ext}}=80\text{mm}$

Annular Shear Rheometer (ASR)

- Local strain measurements from some 10^{-6} to some 10^{-2}
- High stress and strain resolutions
- precise loading conditions
- Temperature control
- Sinusoidal loading up to 10Hz

Focus on small strain



H=40mm, $\phi_{\text{ext}}=105\text{mm}$, th=5mm

Kind of tests and measurements

LVE Theory

- Tension/compression

Axial stress $s_1(t) = s_{01} \sin(\omega t + f)$

Axial strain $e_1(t) = e_{01} \sin(\omega t)$

Radial strain $e_2(t) = -e_{02} \sin(\omega t + f_n)$

Complex Young's modulus

$$E^* = (\sigma_{01} / \varepsilon_{01}) e^{j\phi}$$

Poisson's ratio

$$\nu^* = (\varepsilon_{01} / \varepsilon_{02}) e^{j\phi_\nu}$$



3D approach

- Annular Shear Rheometer

Shear stress $t(t) = t_0 \sin(\omega t + f_t)$

Shear strain $g(t) = g_0 \sin(\omega t)$

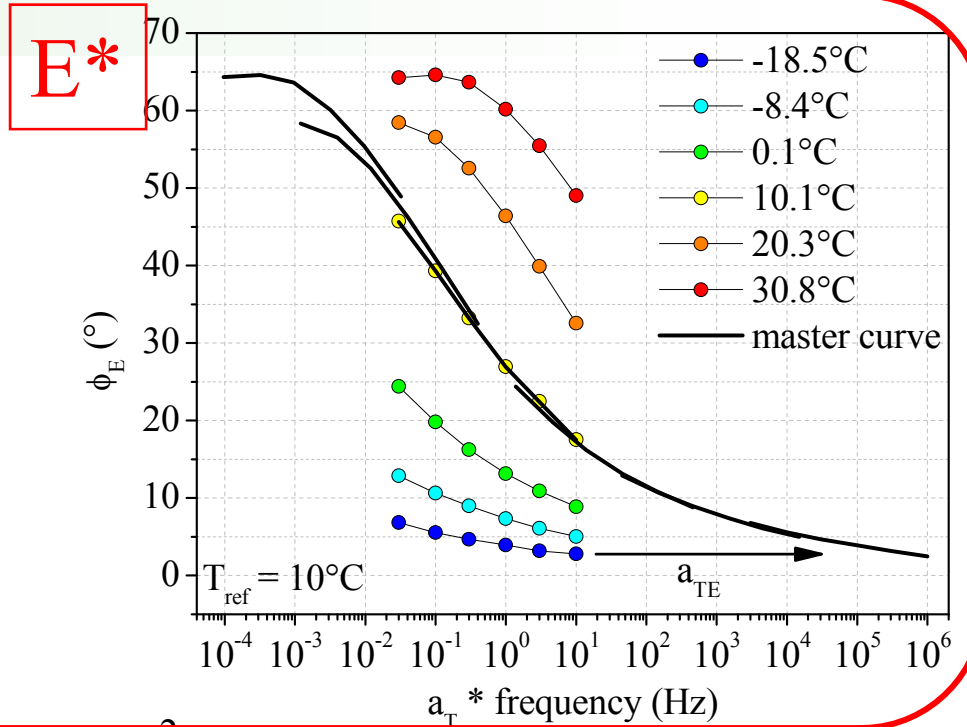
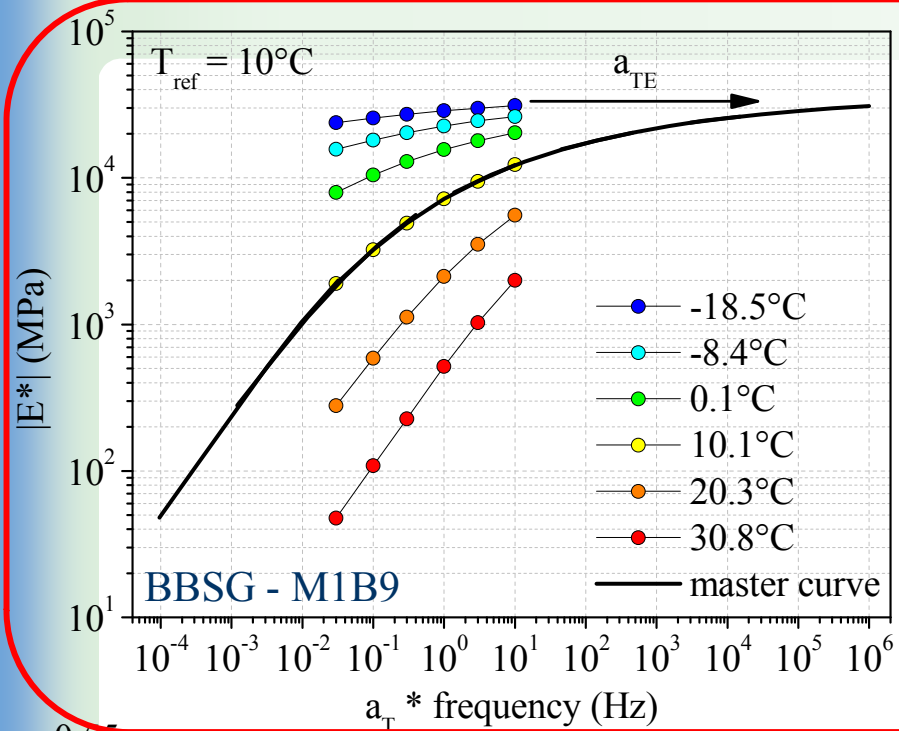
Shear modulus

$$G^* = (\tau_0 / \gamma_0) e^{j\phi_\tau}$$



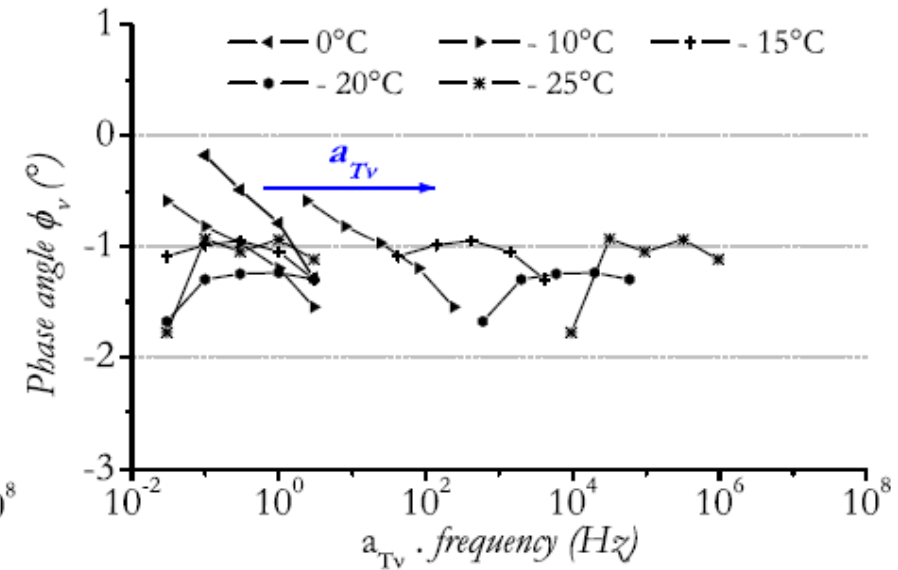
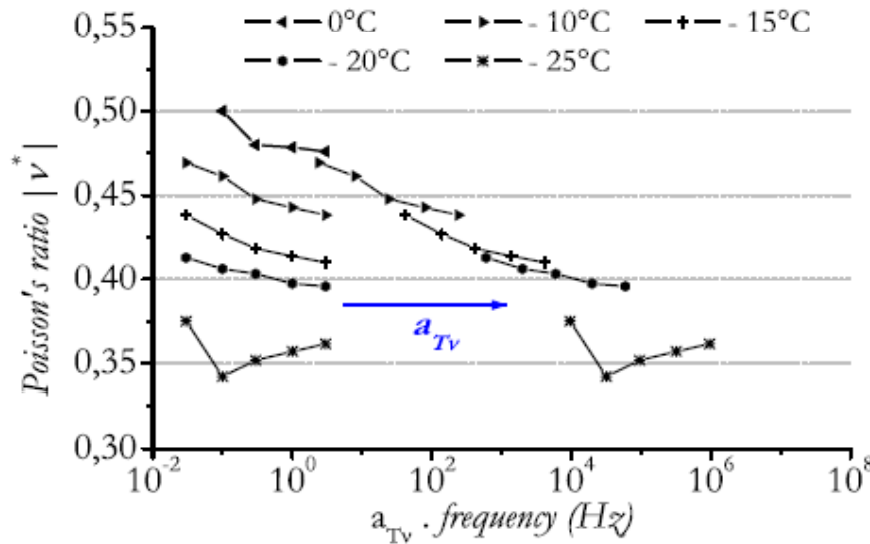
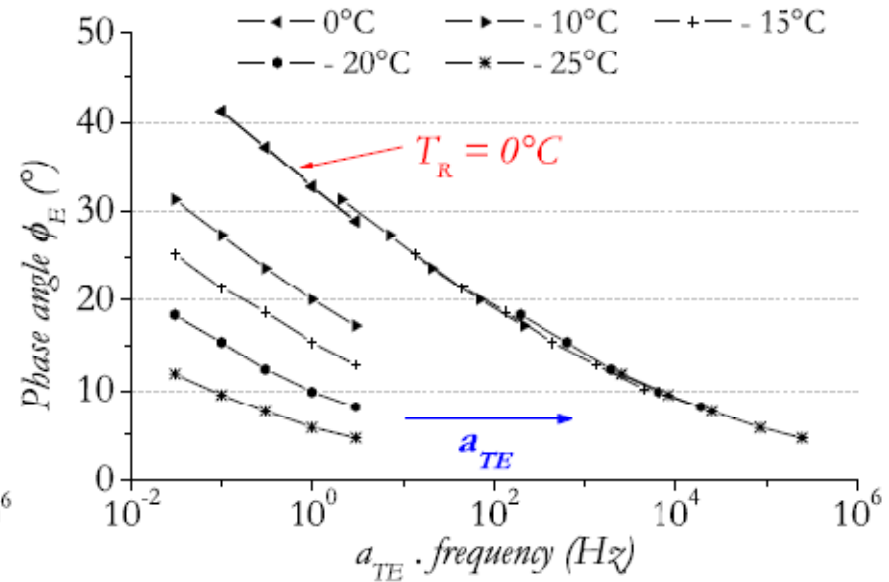
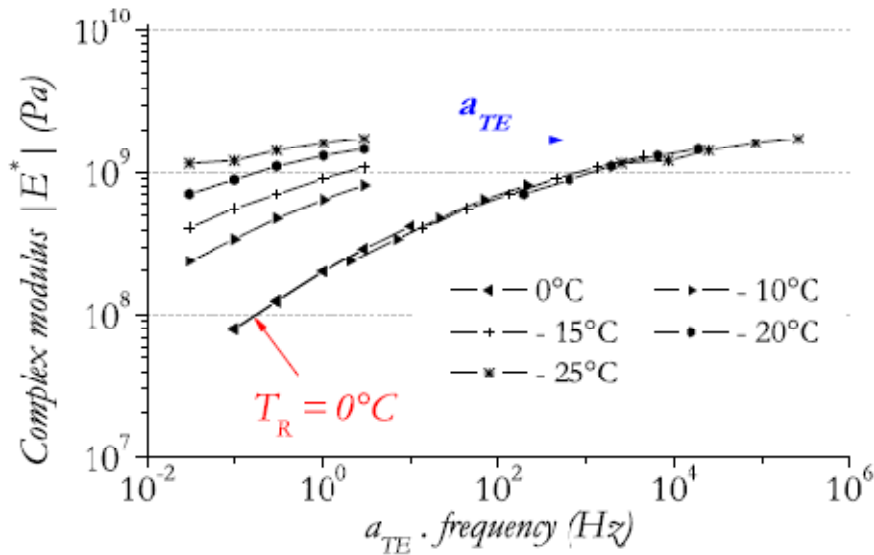
1D approach

*Validation of the time-Temperature
superposition principle
(linear domain)*



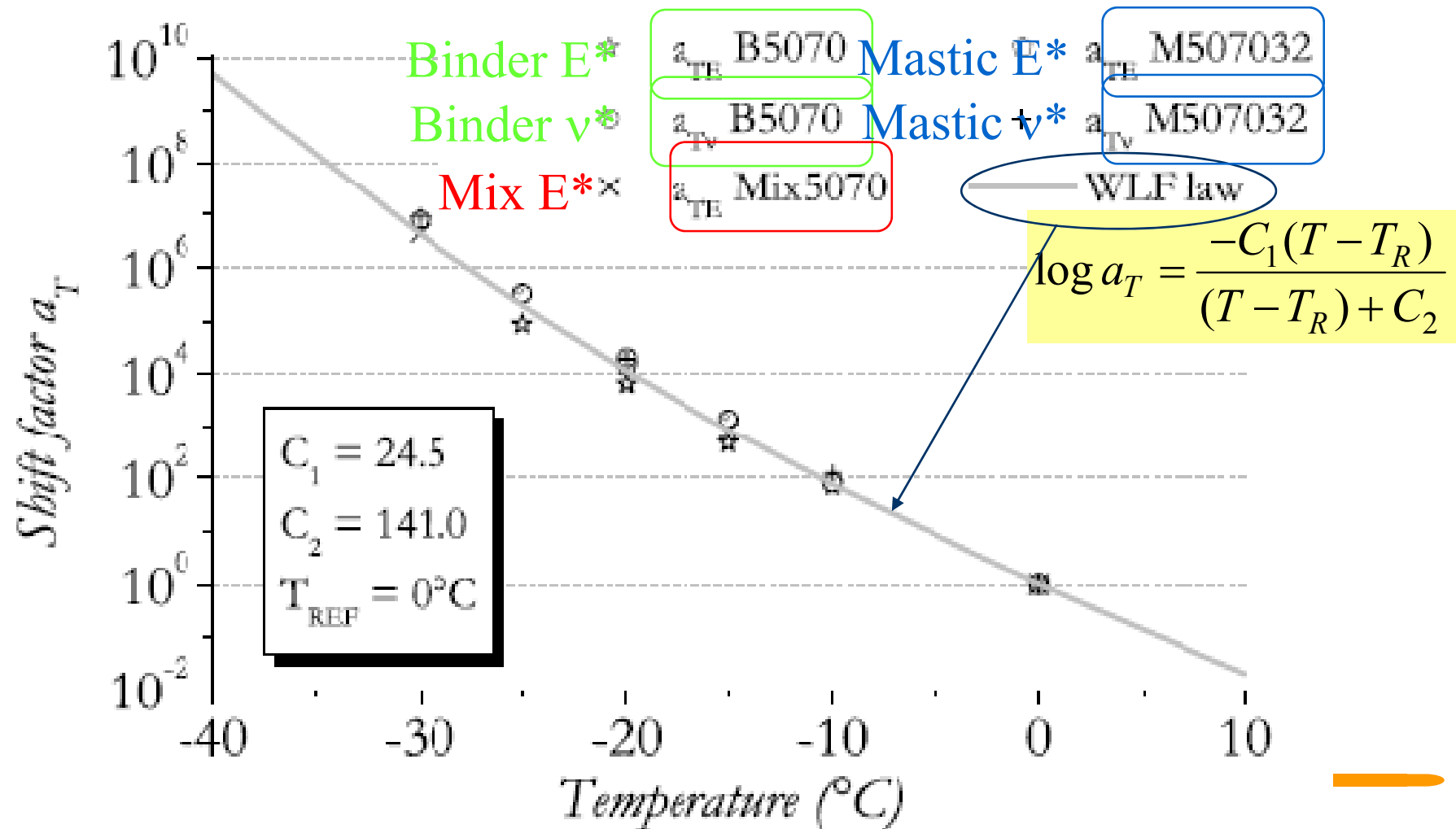
Bituminous mixture: BBSG

Bitumen (B 50/70) : master curves



Shift factor : a_T

Close shift factor for E^* and ν^* ; fixed by the binder

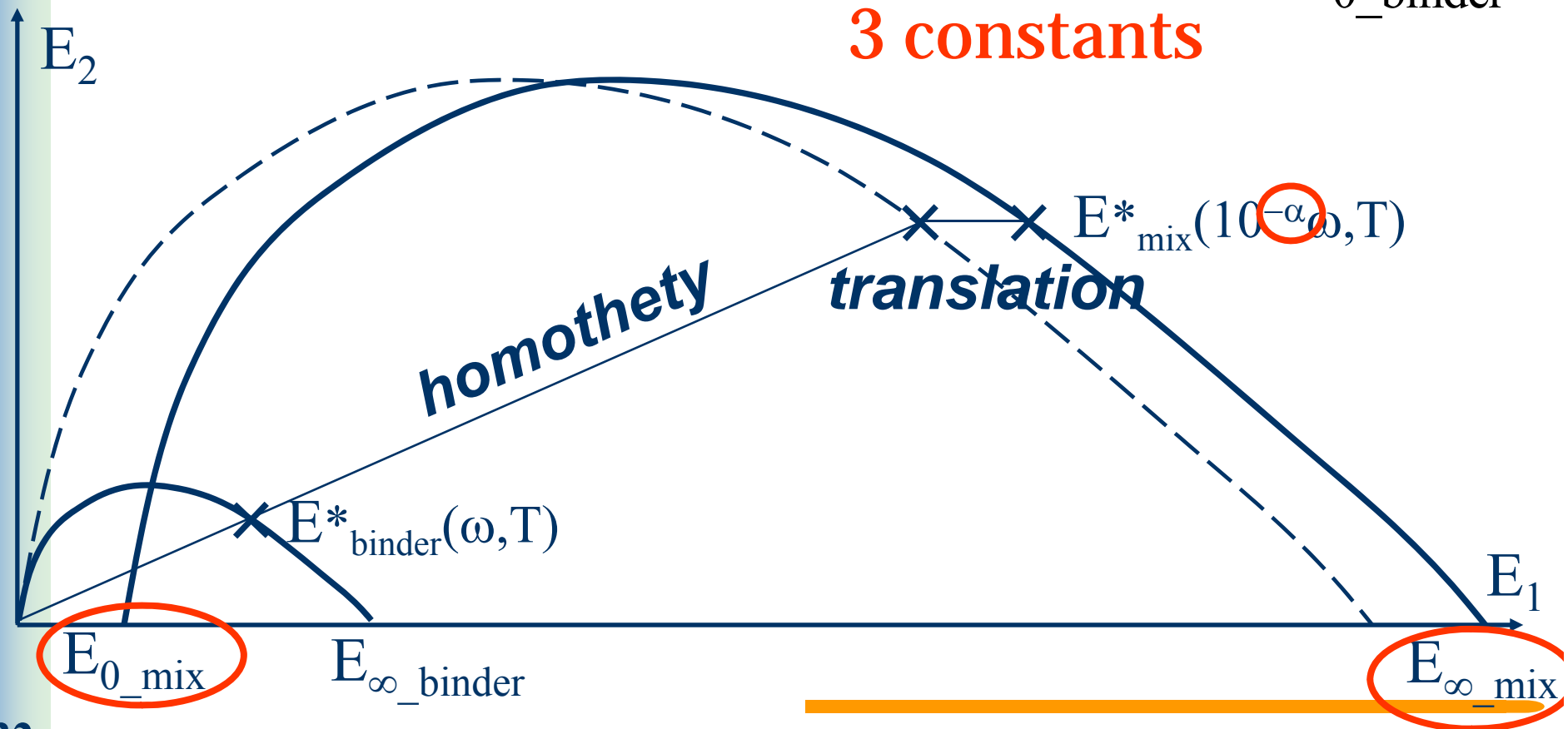


*Prediction of the mix VEL behaviour
from binder*

Prediction of the mix VEL behaviour from binder : complex modulus

$$E_{\text{mix}}^*(\omega, T) = E_{00_mix} + E_{\text{binder}}^*(10^\alpha \omega, T) \frac{E_{0_mix}}{E_{0_binder}}$$

3 constants



Prediction of the mix VEL behaviour from binder : Poisson's ratio

$$\frac{\nu^*(i\omega\tau) - \nu_{00}}{\nu_0 - \nu_{00}} = \frac{E_{mix}^*(i\omega\tau) - E_{00_mix}}{E_{0_mix} - E_{00_mix}}$$

2 constants

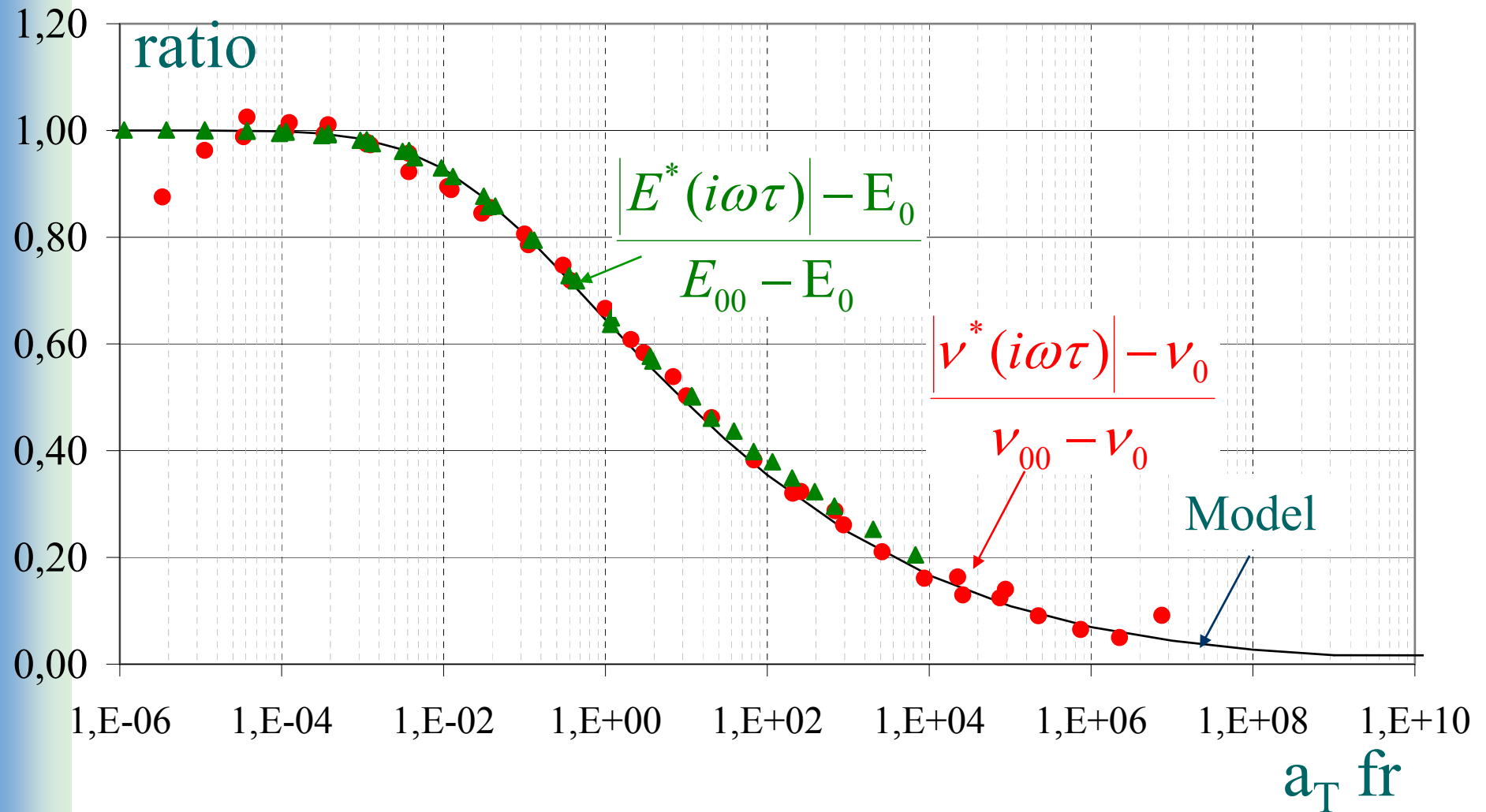
- 5 constants to obtain the 3D mix behaviour from the binder one
- Verified by 2S2P1D (and DBN) if 6 parameters are the same for binder and mix:

$\delta, \eta, h, k, C_1 \text{ \& } C_2$

*Examples of simulations : 2S2P1D &
DBN & link between binder and mix*

Modelling E and ν : BBSG (M1A2)

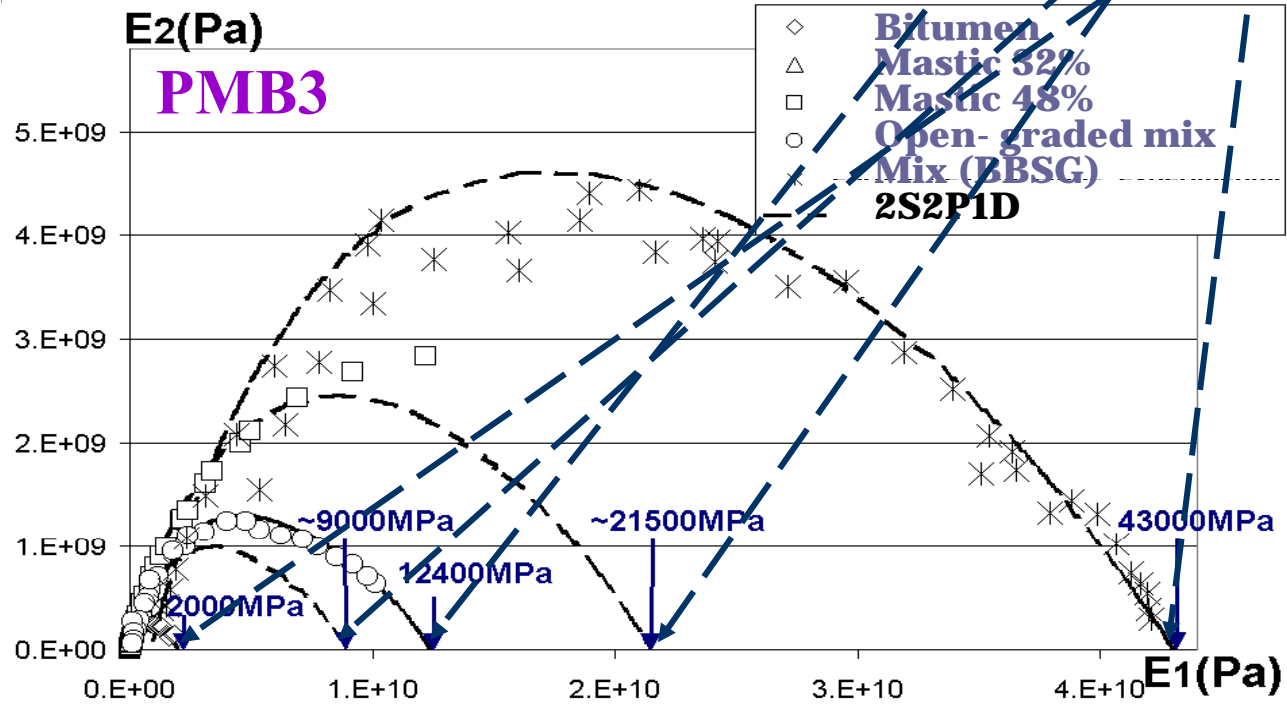
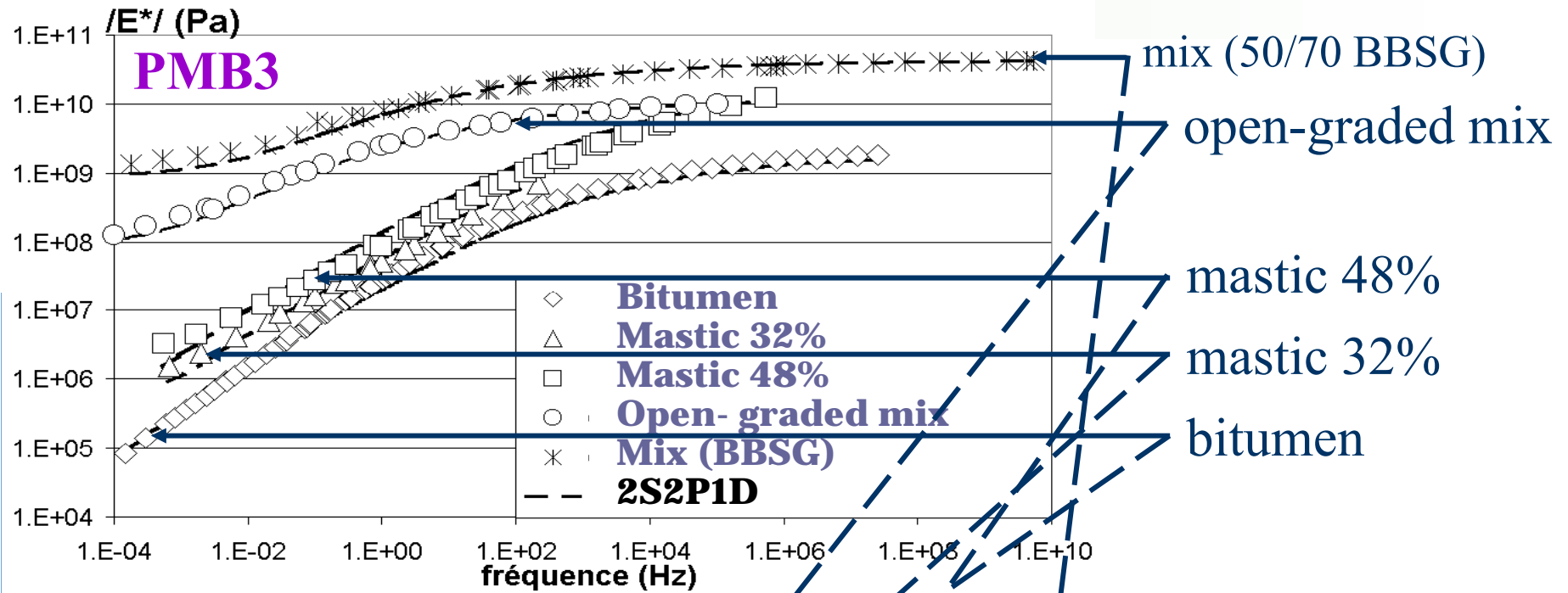
Normalised master curves for E and ν



35

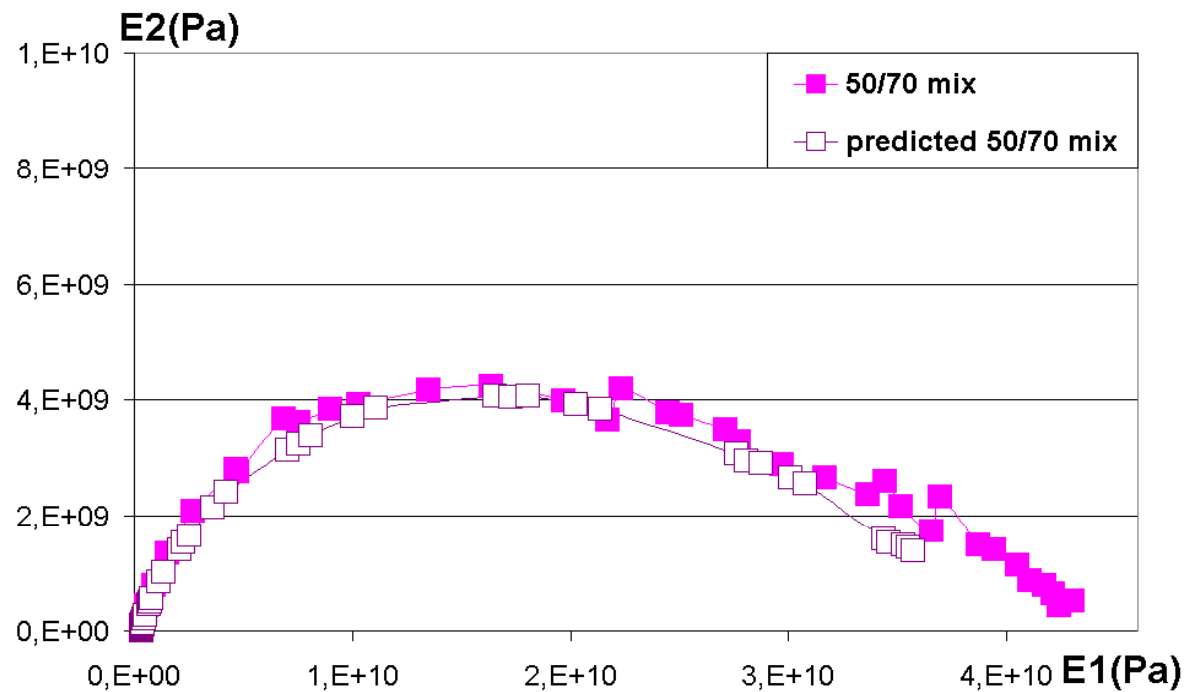
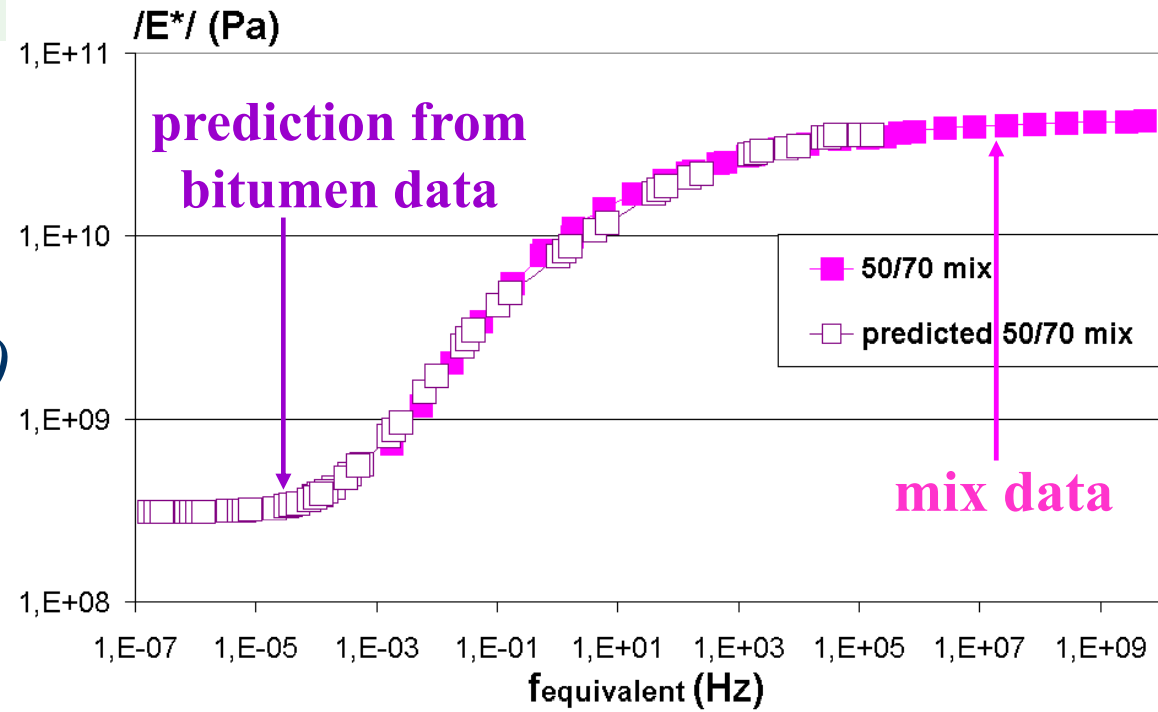
Constants

E0 (MPa)	Einf (MPa)	k	h	Delta	Tau	Beta	ν_0	ν_∞
50	40000	0,21	0,58	1,9	8,00E-01	20	0,42	0,19



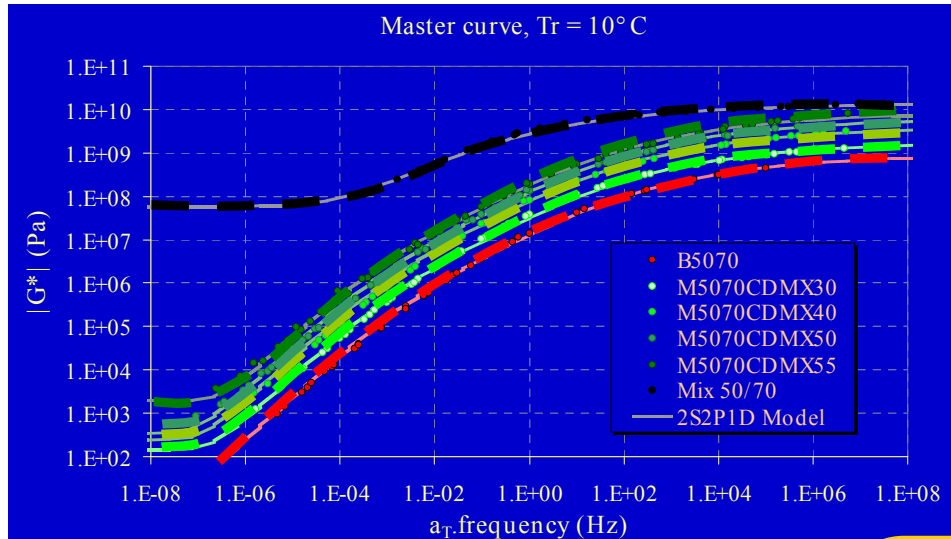
Evolution with granular concentration

Prediction (from transformation)



2S2P1D Parameters

$$G^*(\omega) = G_0 + \frac{G_\infty - G_0}{1 + \delta(i\omega\tau)^{-k} + (i\omega\tau)^{-h} + (i\omega\beta\tau)^{-1}}$$



4 constant parameters:

k, h, δ, β

Only 3 parameters
function of the aggregate
structure (filler
concentration,):

G_0, G_∞, τ_0

Material	G_0 (Pa)	G_∞ (Pa)	k	h	δ	$\tau_0 = \tau$ (10°C)	β
B5070	0	9.50E+08	0.21	0.55	2.3	9.18E-05	450
M5070U100 μ 30	150	1.85E+09	0.21	0.55	2.3	1.40E-04	450
M5070U100 μ 40	250	4.10E+09	0.21	0.55	2.3	1.73E-04	450
M5070U100 μ 50	350	6.30E+09	0.21	0.55	2.3	1.83E-04	450
M5070U100 μ 55	2000	8.80E+09	0.21	0.55	2.3	2.18E-04	450
Mix 50/70	6.00E+07	1.40E+10	0.21	0.55	2.3	7.00E-02	450

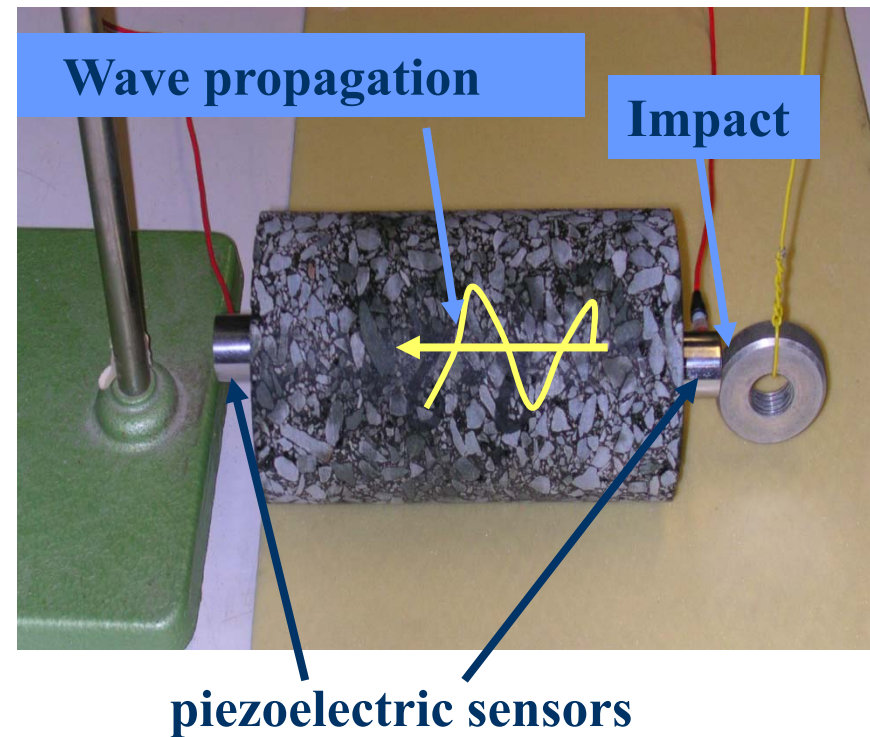
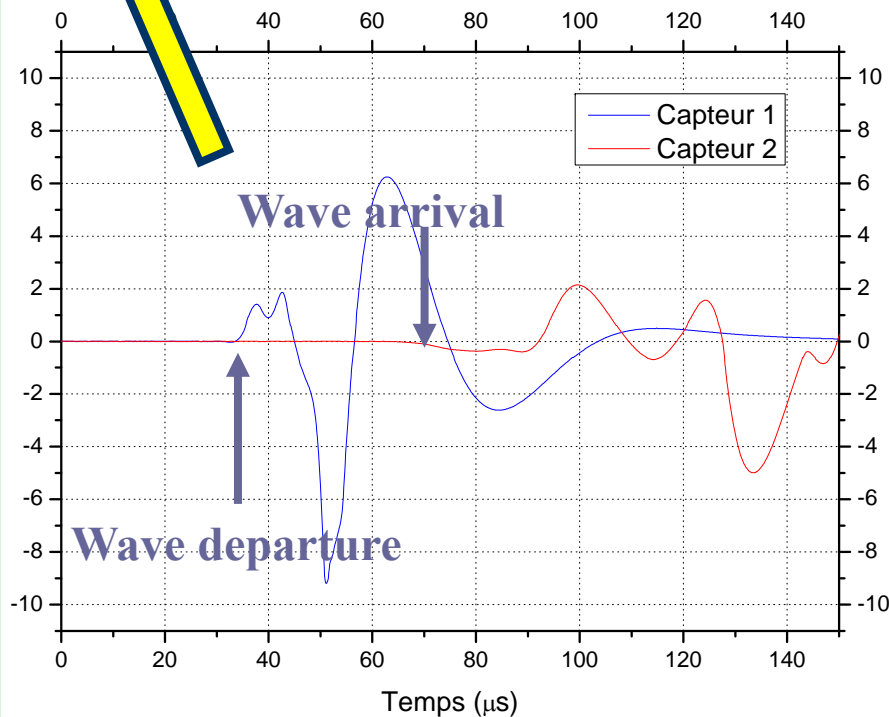
Generalisation of the Time-Temperature superposition principle

- Linear domain
 - Different experimental validations already shown: validity in 3 dim & same a_T for E^* and ν^*
- Non linear domain
 - Cyclic compression & cyclic tension test
- High frequencies
 - Back analysis of wave propagation (linear domain)

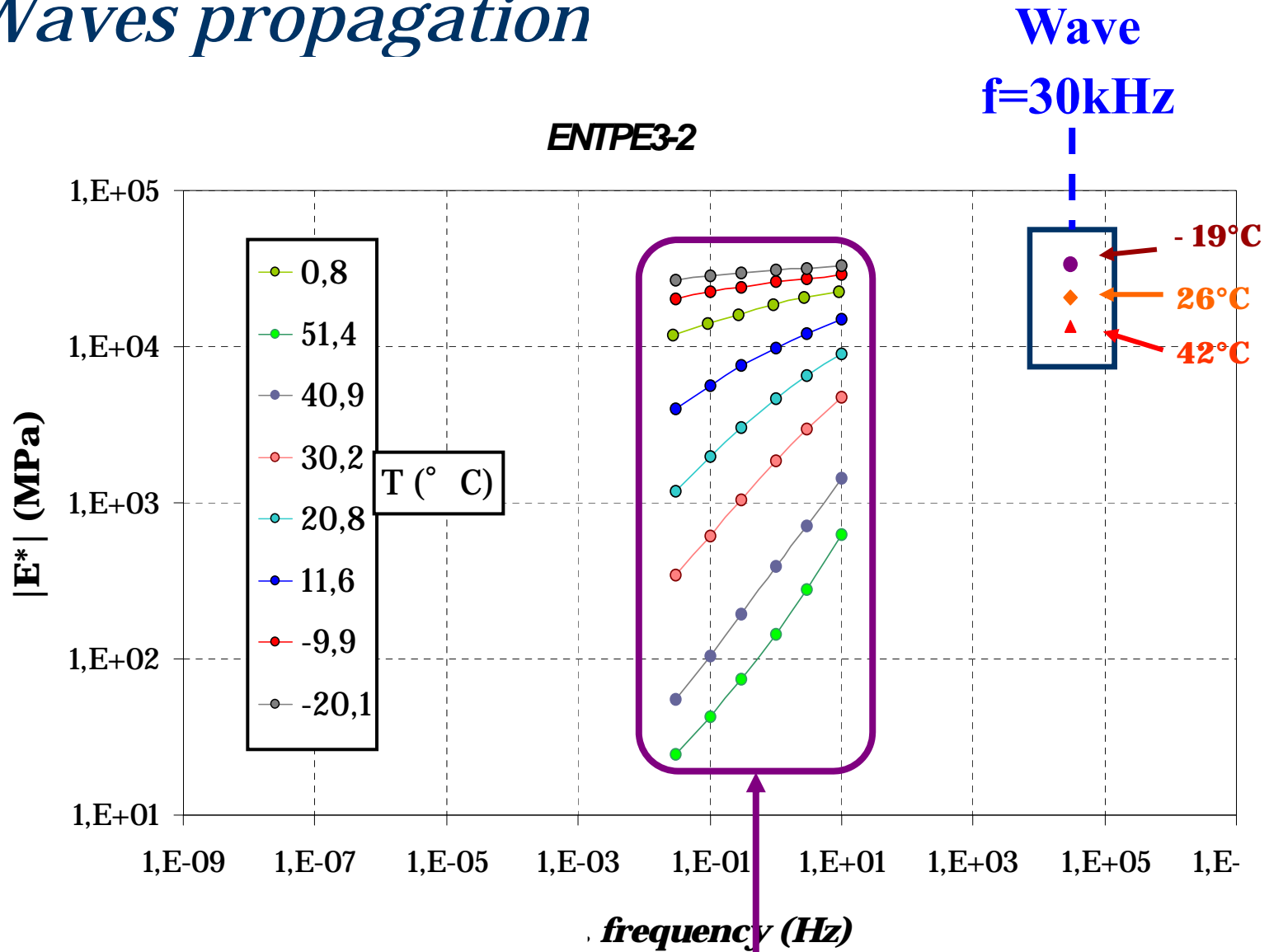
Waves propagation

- Back analysis

$$C_p = \frac{1}{\cos\left(\frac{\phi}{2}\right)} \sqrt{\frac{(1-\nu) |E^*|}{(1+\nu)(1-2\nu)\rho}}$$



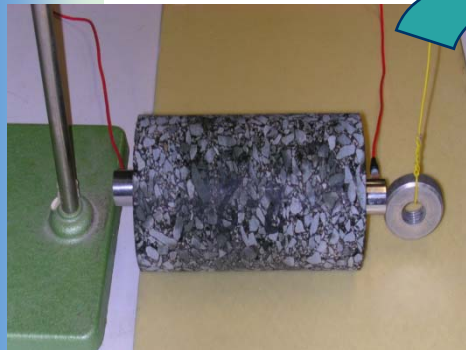
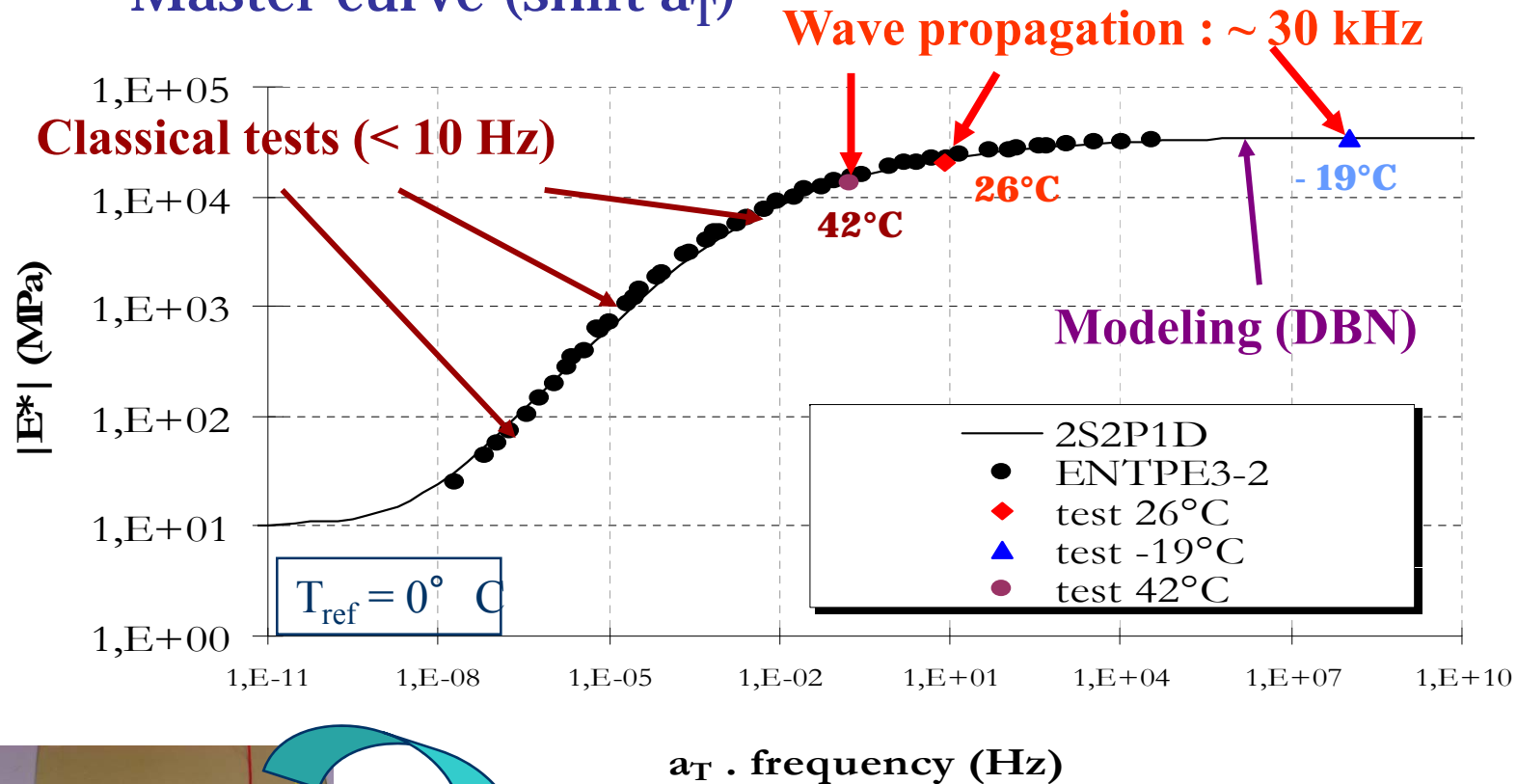
Waves propagation



Complex modulus test

Wave propagation: master curve

- Master curve (shift a_T)



Moduli & Validation of the t-T at high frequency

*Importance to chose an appropriate
model for simulation*

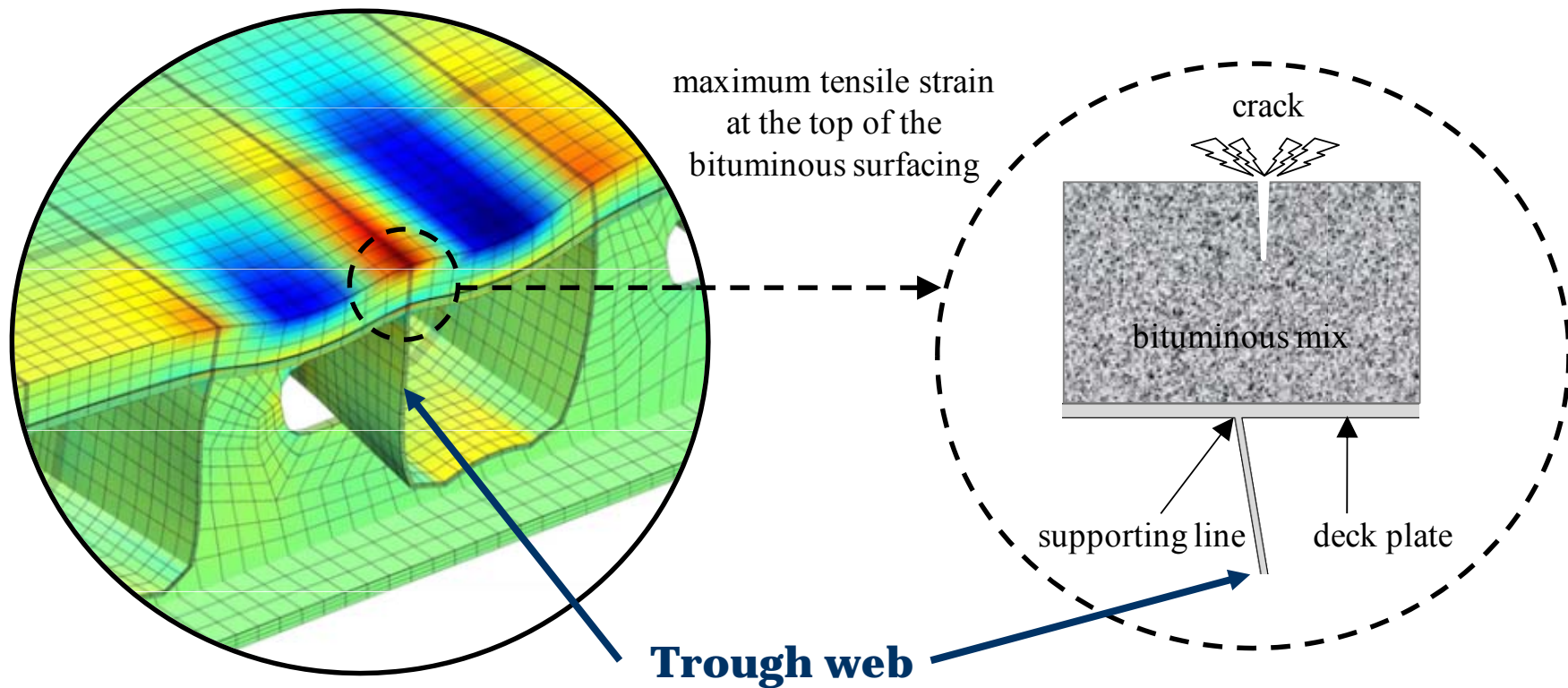
→ Elasticity versus Viscoelasticity

Elasticity versus Viscoelasticity

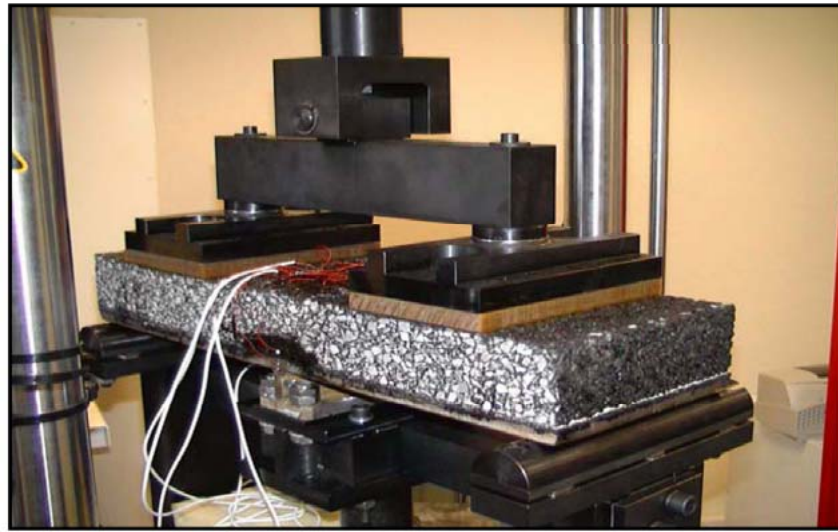
- FEM calculation of the 5 point bending test (french standard NF P 98-286) use to study fatigue of mixtures surfacing on orthotropic steel briges



Identified problem for ortotropic bridges

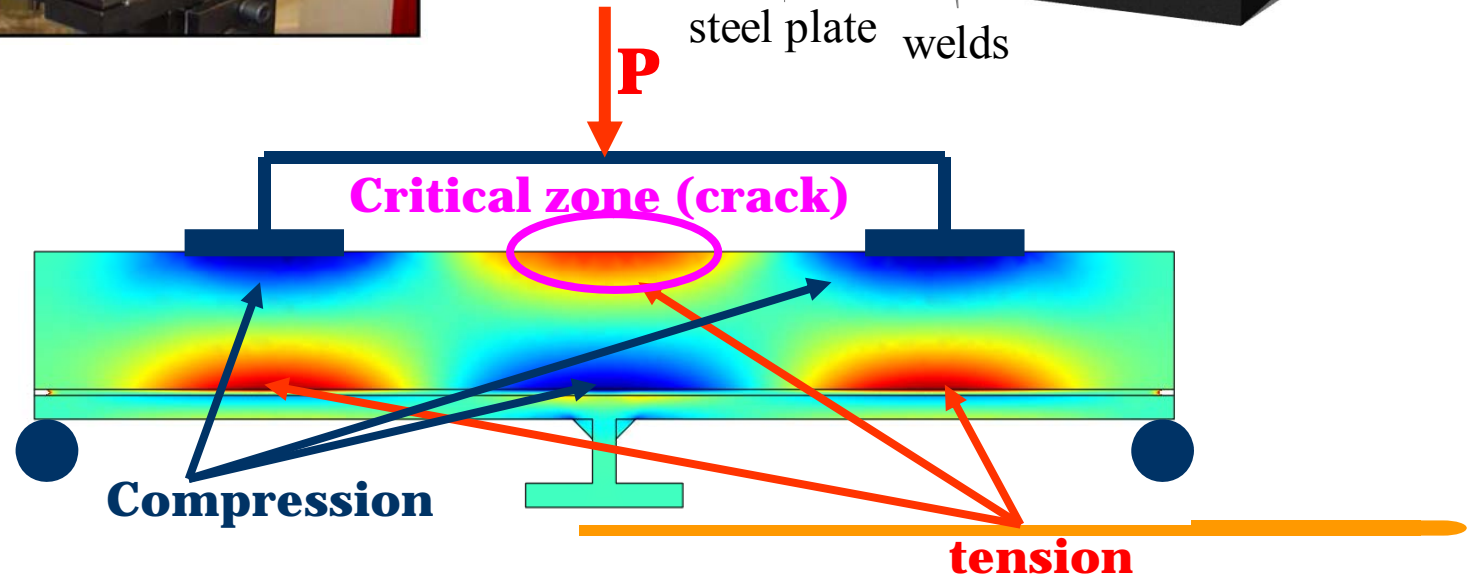
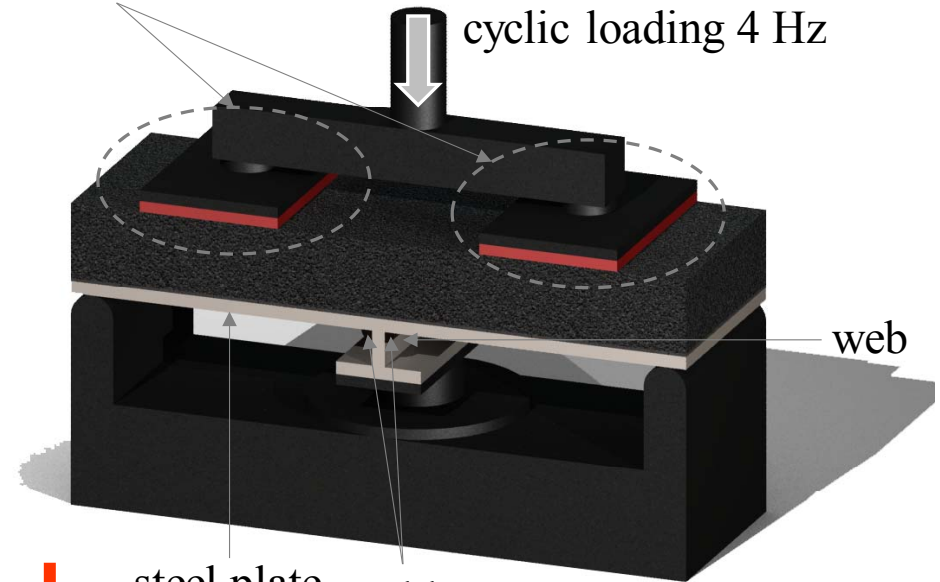


5 point bending test (principle)



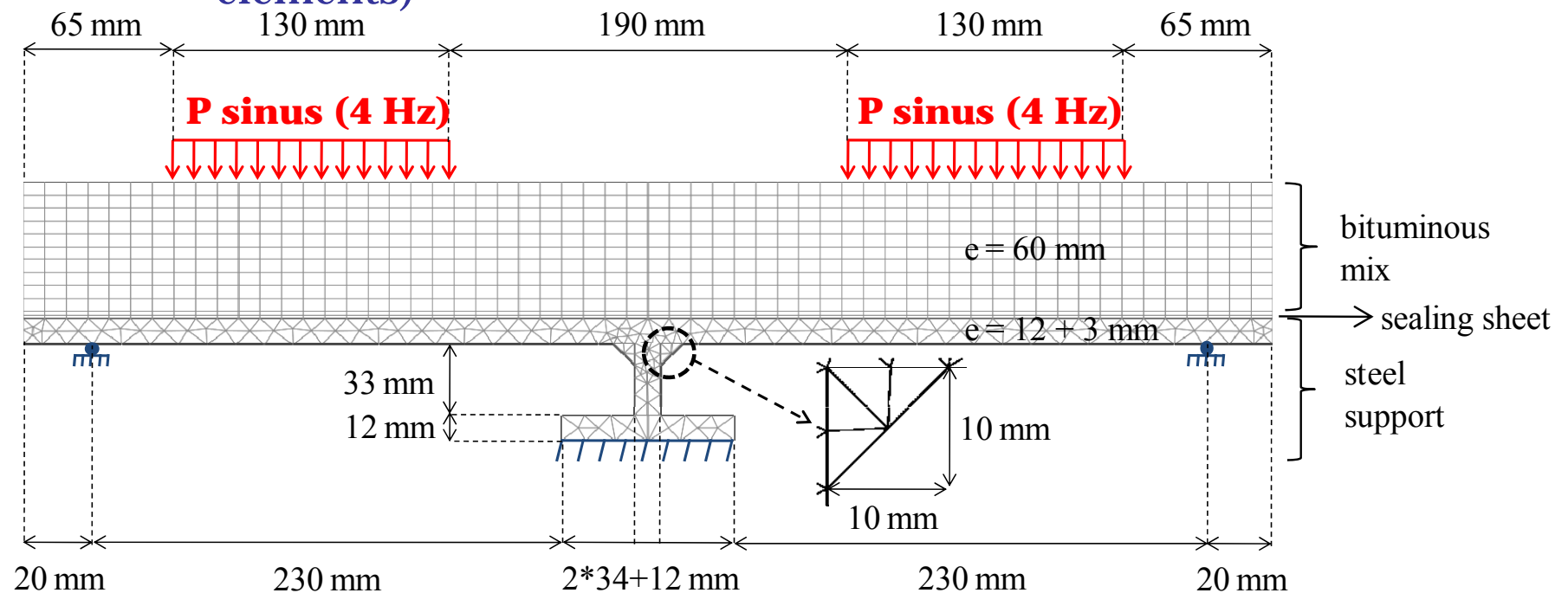
Shoes with rubber

cyclic loading 4 Hz



FEM Calculation

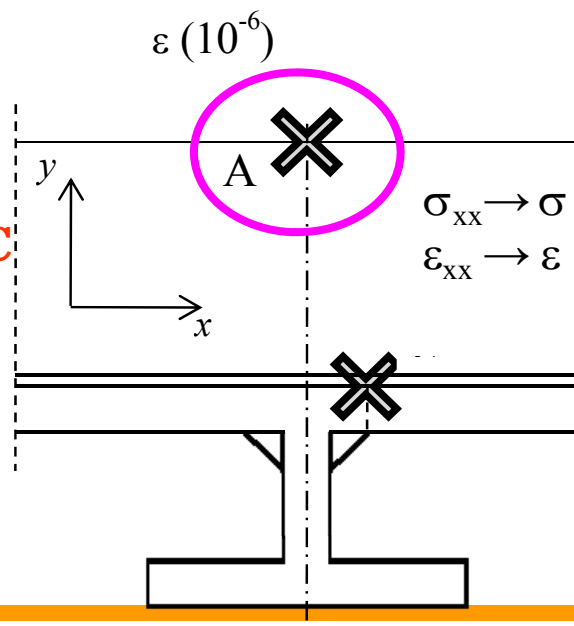
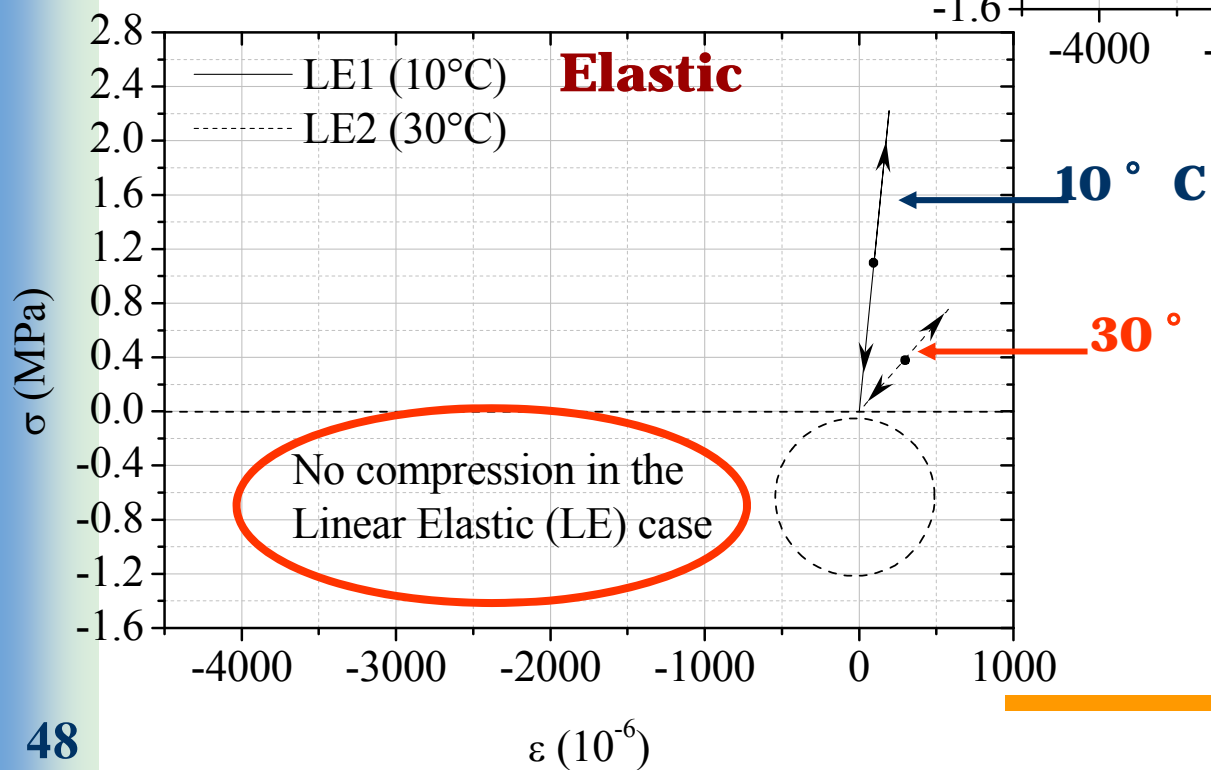
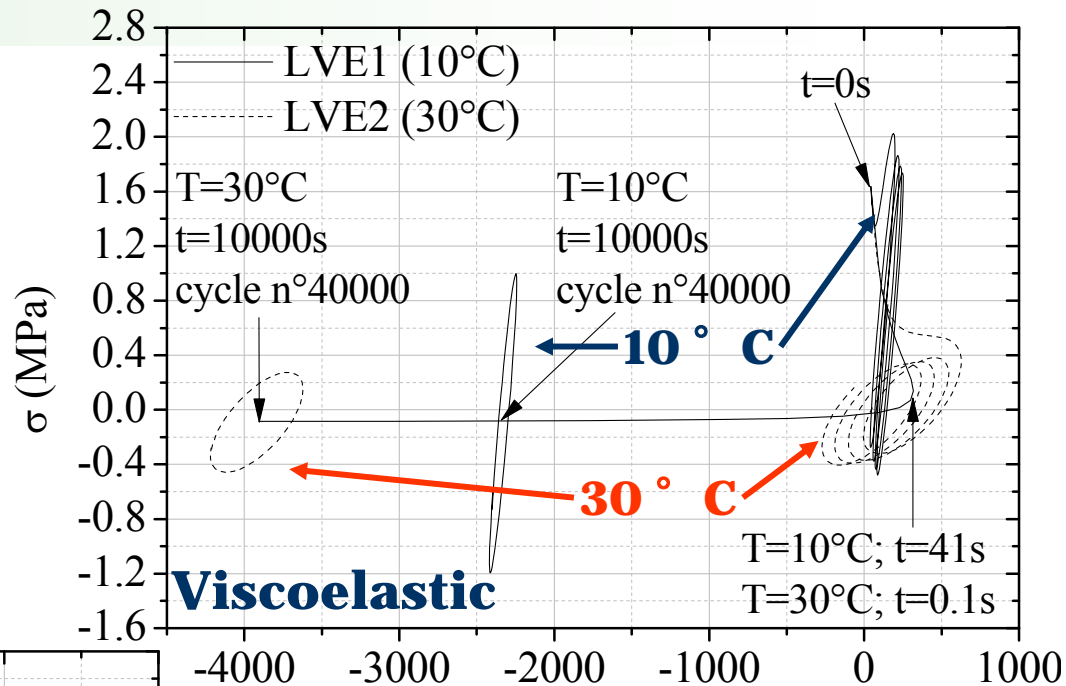
- Steel and sealing sheet : linear elastic isotropic
- Mix surfacing (isotropic)
 - Elastic (modulus fixed by temperature and frequency)
 - Viscoelastic with ν (Poisson's ratio) constant
 - Viscoelastic with ν function of time (DBN model with 20 elements)



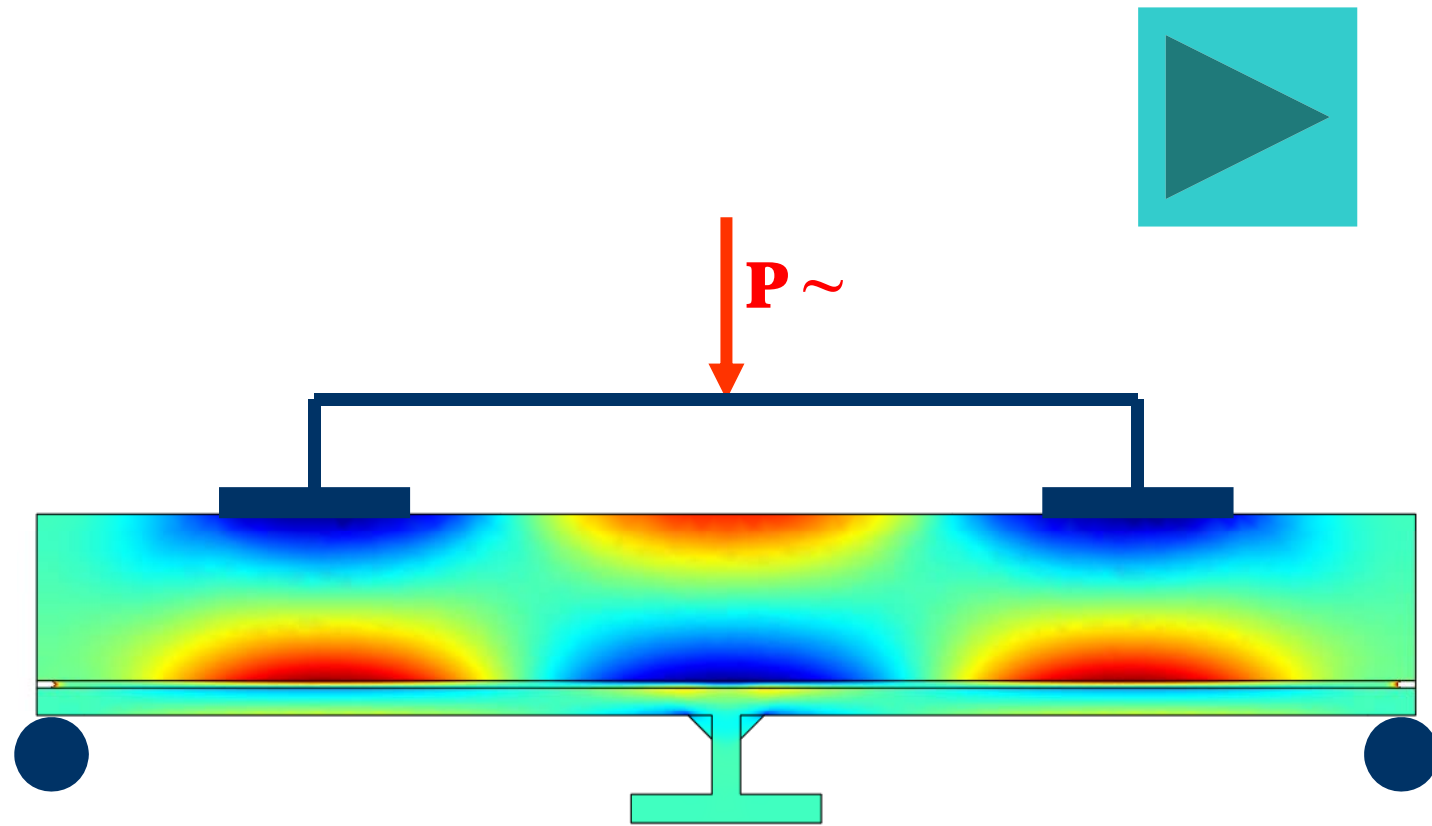
FEM Calculation

Calculation at point « A »

Big difference



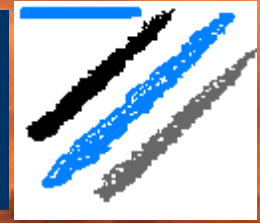
FEM Calculation (30° C)



- *Powerful constitutive law for bituminous materials &*
- *Implementation for road calculation and design*

 **A research challenge**

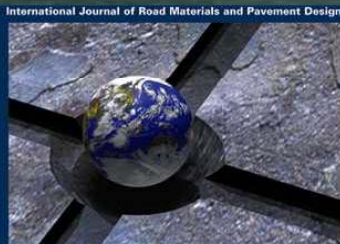
With important practical implication



Thank You

どうも ありがとう！
Merci

**Road
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**Hervé Di Benedetto
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